

Aalborg Universitet

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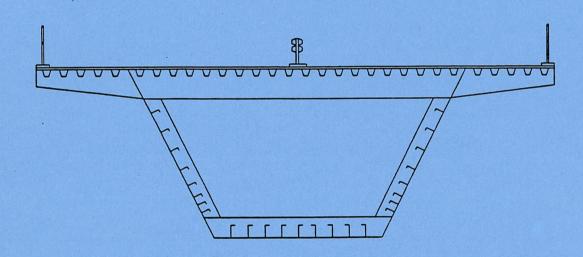
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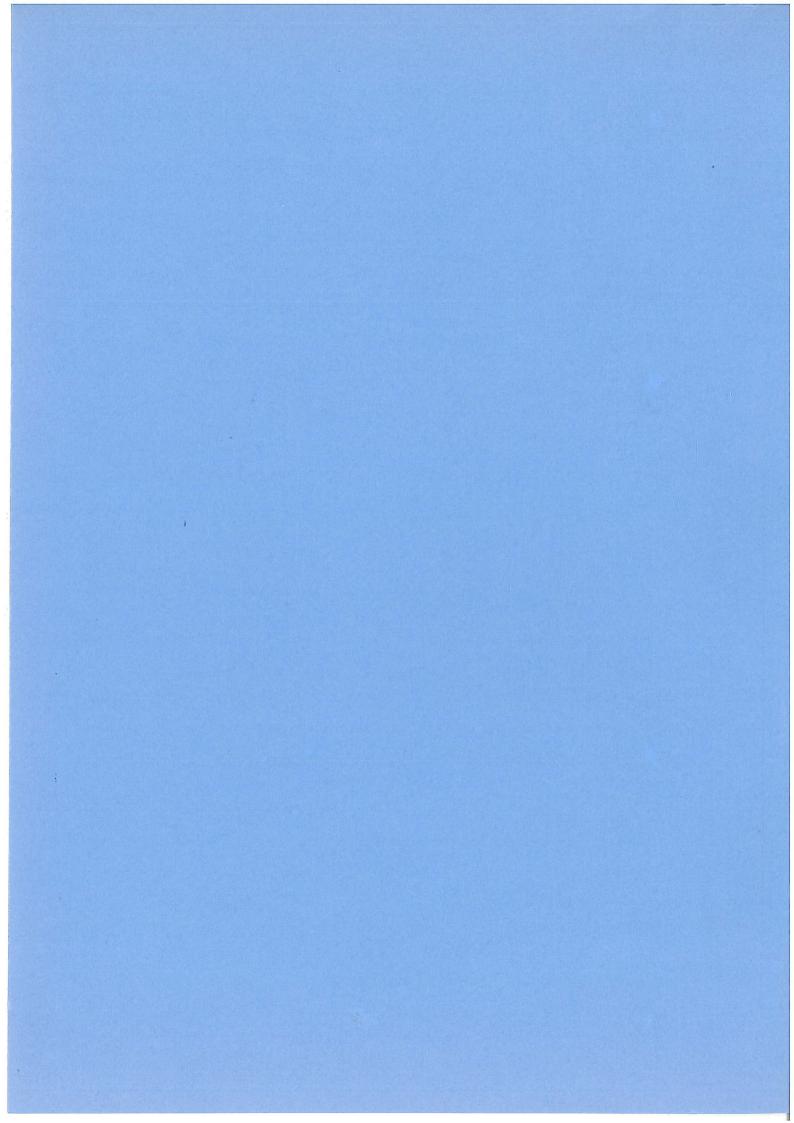
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FIGURHÆFTE

STORE STÅLBROER - Bjælkebroer samt skråstags- og hængebroer ESDEP-UDDRAG v/A. ALBERTSEN AUGUST 1994 ISSN 0902-8005 U9408



Dette hæfte knytter sig til NOTE U9407, der er et teksthæfte med samme titel.

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13B.1	Conceptual	CHOICE

- 15B.2 Actions on Bridges
- 15B.3 Bridge Decks
- 15B.4 Plate Girder and Beam Design
- 15B.5 Truss Bridges
- 15B.6 Box Girder Bridges
- 15B.8 Cable Stayed Bridges
- 15B.9 Suspension Bridges
- 15B.10 Bridge Equipment
- 8.5.1 Design of Box Girder Bridges
- 8.5.2 Advanced Methods for Box Girder Bridges

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LECTURE 15B.1 Conceptual Choice

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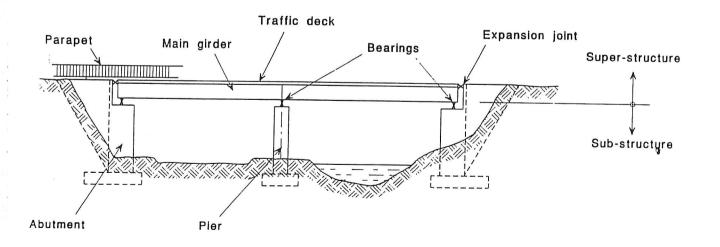
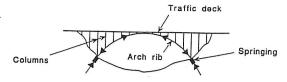
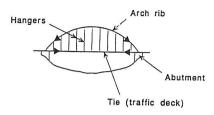


Figure 1 Elevation of typical girder bridge

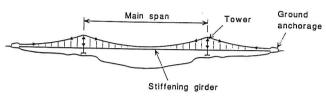




(a) Fixed arch bridge



(b) Tied arch bridge



(c) Suspension bridge

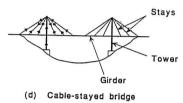




Figure 2 Types of bridge that carry load mainly by axial forces.

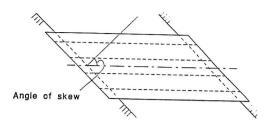


Figure 3 Typical plan view of skew bridge



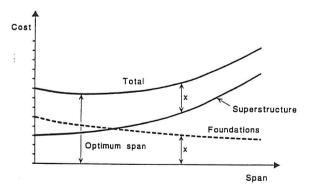


Figure 4 Bridge costs (diagrammatic)

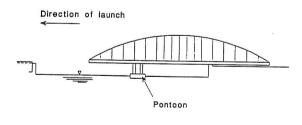


Figure 5 Typical launching arrangement using pontoon





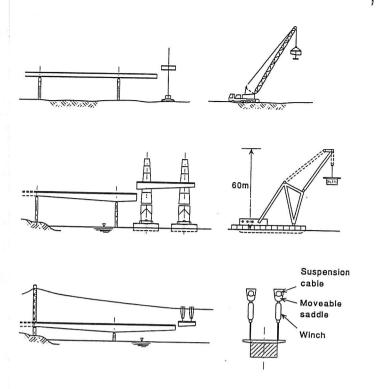
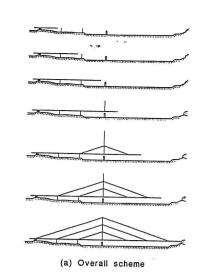
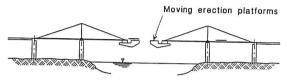


Figure 6 Typical methods of lifting bridge sections





50T moving derricks



(b) Options for positioning segments Figure 7 Typical cantilever erection of a cable-stayed bridge.



Lecture 15B.1

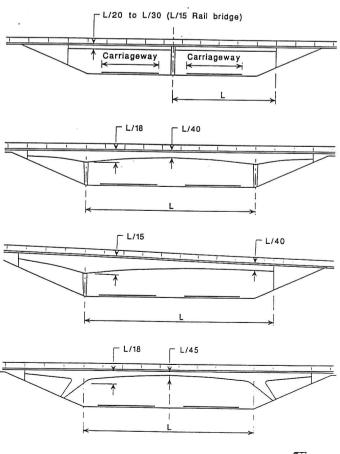


Figure 8 Typical elevations of highway overbridges.



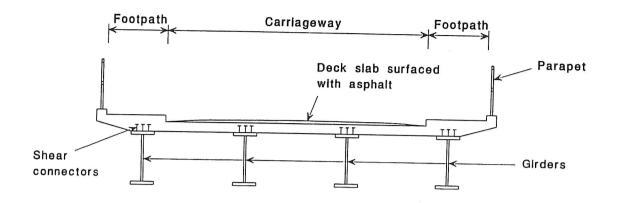


Figure 9 Typical cross-section of a single carriageway highway bridge

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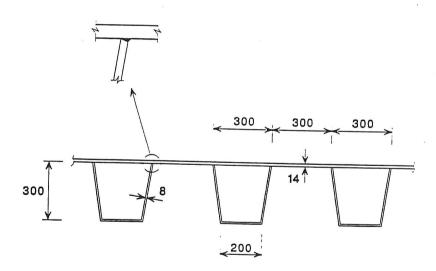
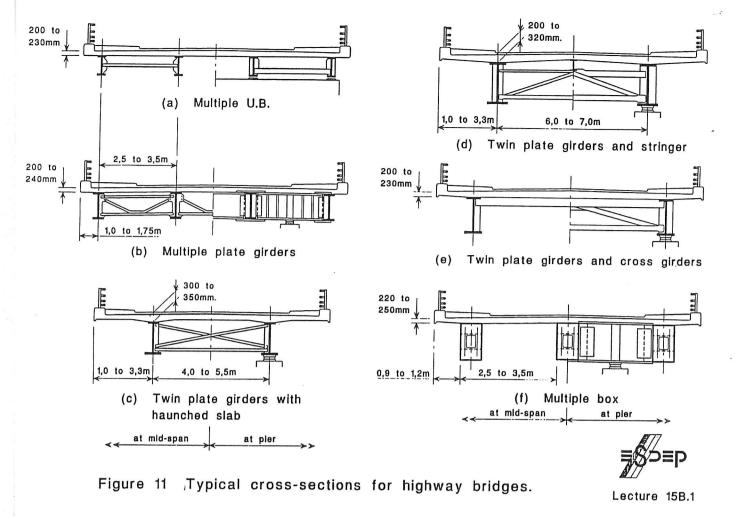
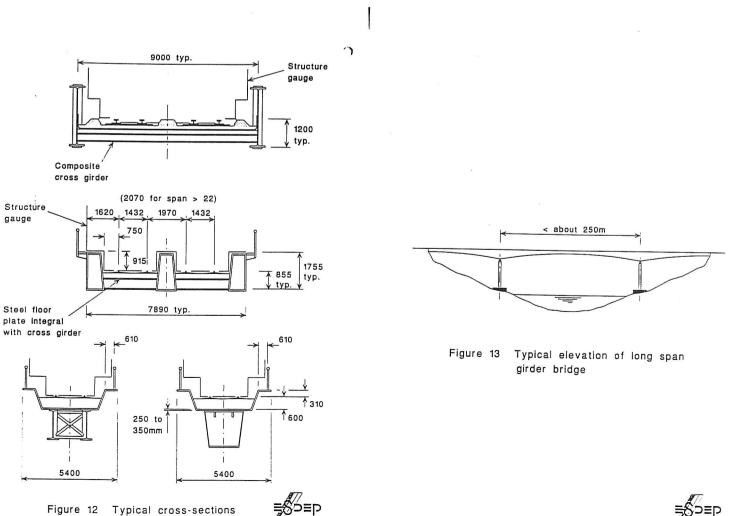


Figure 10 Orthotropic steel deck.







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for railway bridges

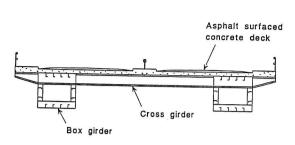


Figure 14 Typical cross-section of wide, long span, girder bridge.

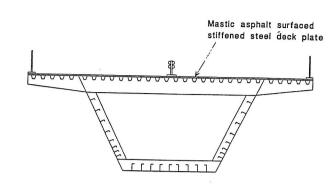


Figure 15 Typical cross-section of box girder long span bridge





t=12mm



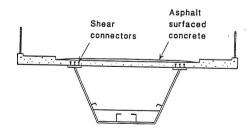
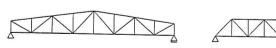


Figure 16 Typical open top composite box-girder bridge



(i) Traditional design with high fabrication content

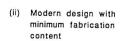


Figure 17 Changes in economic design



Lecture 15B.1



LECTURE 15B.2 Actions on Bridges

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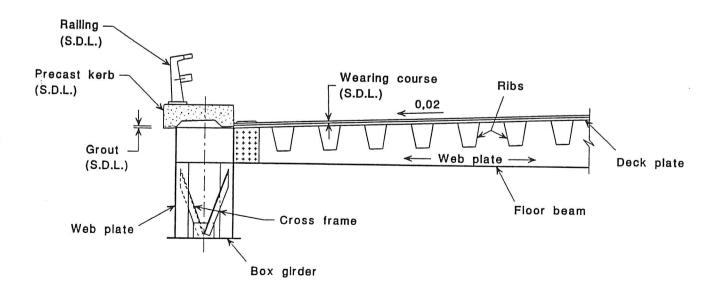


Figure 1 Dead load and superimposed dead load (S.D.L.)



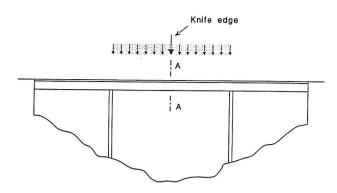


Figure 2 Location of distributed and knife edge loads to produce maximum mid-span moment.

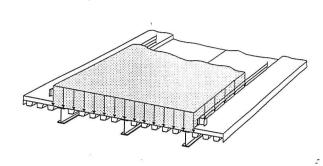


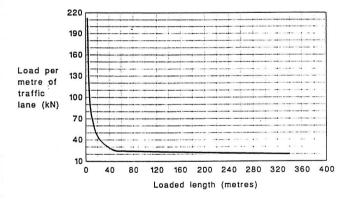
Figure 3 Uniformly distributed load on traffic lanes.



Lecture 15B.2



Lecture 15B.2



Note: HA loading consists of a uniformly distributed load, expressed in kN per linear metre of traffic lane and a knife edge load of 120kN applied across the width of the lane.

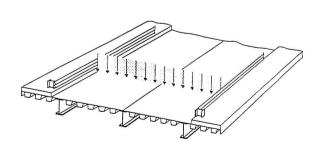


Figure 5 Knife-edge load.

Figure 4 Variation of type HA loading with loaded length (U.K. Department of Transport)



Lecture 15B 2

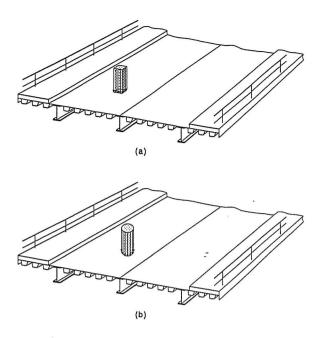


Figure 6 Single wheel loads



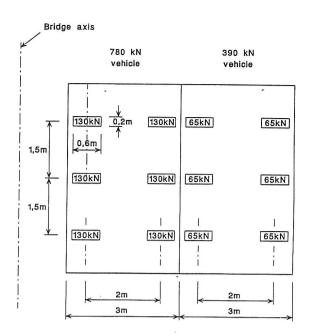
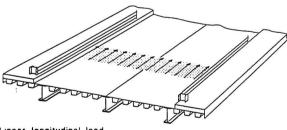
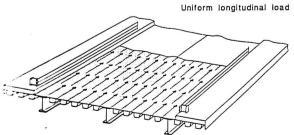


Figure 7 Concentrated wheel loads on bridge decks, according to load specifications by the Danish Road Directorate.





Linear longitudinal load



or

Figure 8 Longitudinal tractive forces.

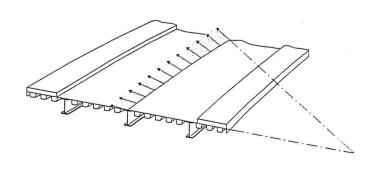


Figure 9 Centrifugal forces.



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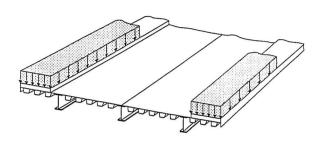


Figure 10 Sidewalk distributed load.

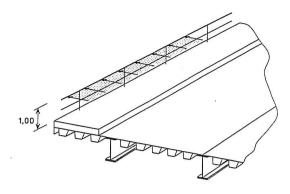


Figure 11 Distributed load on parapet.





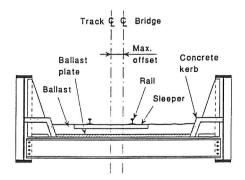


Figure 12 Typical section of a through-girder railroad bridge (curved track)

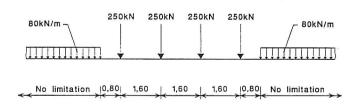


Figure 13 Typical train load.





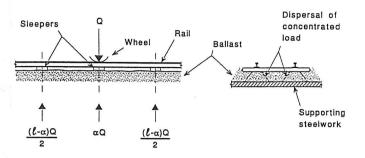


Figure 14 Dispersal of wheel loads.

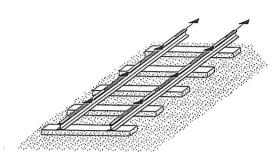


Figure 15 Longitudinal tractive forces.





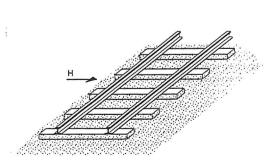


Figure 16 Horizontal force due to nosing

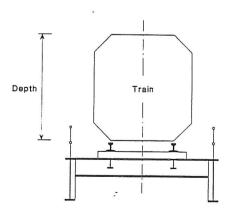


Figure 17 Depth of exposed area for wind actions.



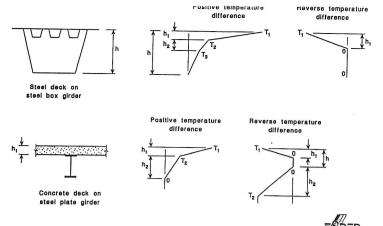


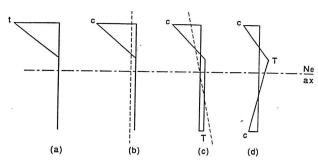
Figure 18 Examples of temperature differences.

Lecture 15B.2

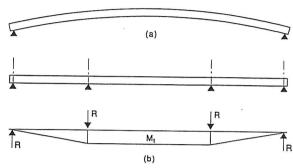
Deck - most of mobile mass - M Fixed Fixed Mobile Mobile supports supports support support Columns - little mobile mass Lateral view Equivalent static forces Transverse section k = rigidity ofcolumns Dynamic model of bridge

Figure 20 Example of bridge with simple dynamic behaviour.



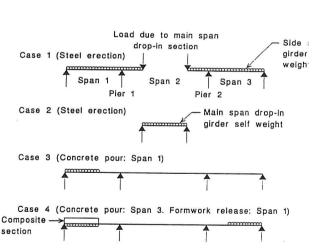


- a) Non-linear temperature distribution.
- b) Equivalent "thermal" stresses if thermal effects fully restraine
- c) Thermal stress distribution if axial restraint removed.
- d) Thermal stress distribution if curvature restraint also removec
 - (i) Primary thermal stresses.



- a) Curved shape of simply supported beam.
- b) Secondary moments and reactions in beam continuous over four supports.
 - ii) Secondary thermal effects.

Figure 19. Structural effects of non-linear temperature distributions.



Case 5 (Concrete pour: Span 2. Formwork release: Span 3)

Case 6 (Concrete pour: Pier 1. Formwork release: Span 2)

Case 7 (Concrete pour: Pier 2. Formwork release: Pier 1) Cracked composite section

Case 8 (Superimposed dead load. Formwork release: Pier 2)

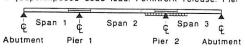


Figure 21 Construction load cases for a three-span composite highway bridge.



LECTURE 15B.3 Bridge Decks

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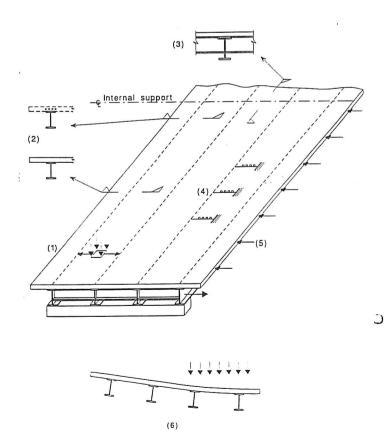
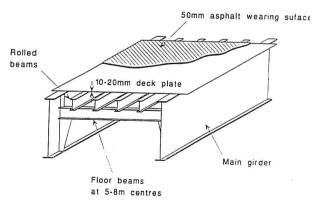


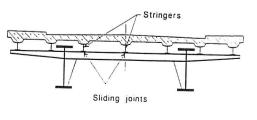
Figure 1 Structural actions of a highway bridge deck.



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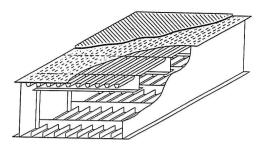
(a) Typical battledeck floor in steel



(b) Concrete slab

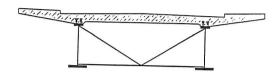
Figure 2 Early highway bridge decks.





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(a) Orthotropic steel deck

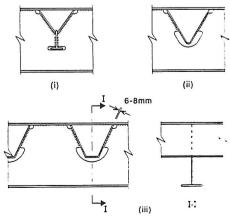


(b) Composite slab

Figure 3 Modern highway bridge decks.



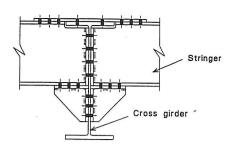
(a) Open, torsionally weak stiffeners



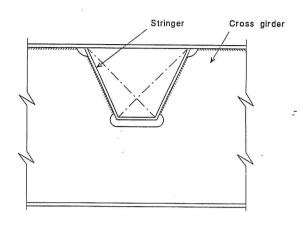
(b) Closed, torsionally stiff stiffeners

Figure 5 Stiffeners (stringers) for orthotropic steel decks





(a) Early bolted or riveted construction



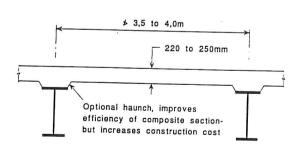
(b) Modern welded construction

Note:- Different orientation of diagrams to illustrate connections

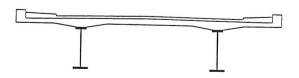
Figure 4 Stringer / Cross girder intersections



Lecture 156.3



(a) Multi girder or cross girder bridge



(b) Twin girder bridge without cross girders

Figure 6 Typical proportions for reinforced concrete highway bridge decks.



Lecture 15B 3

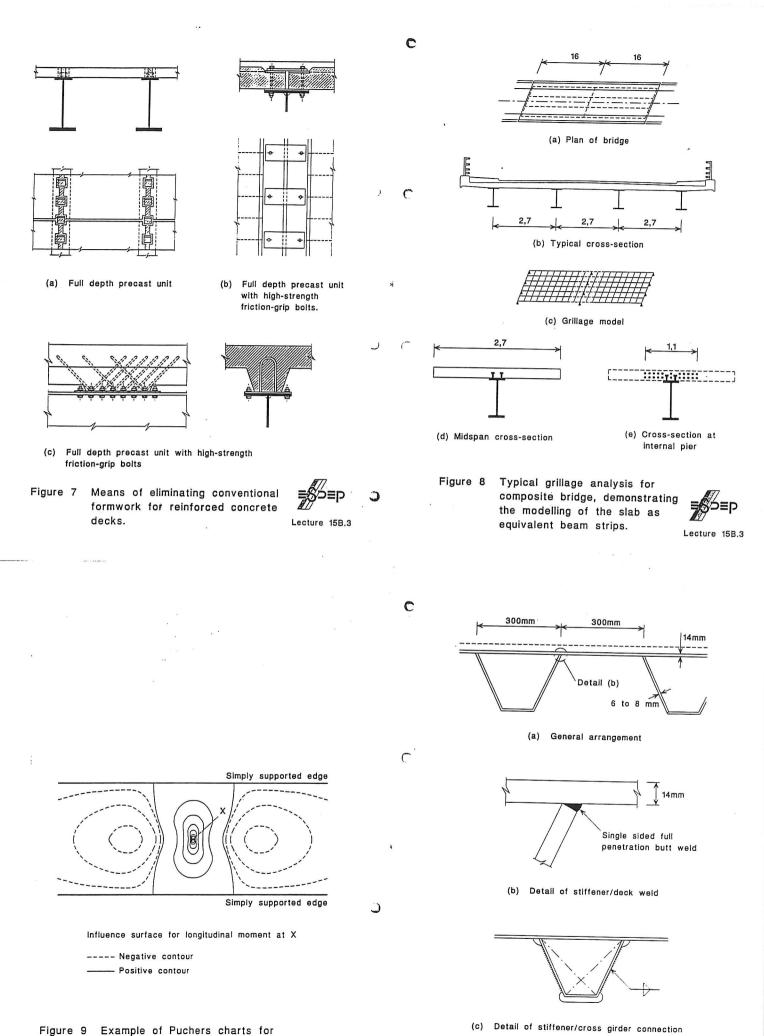
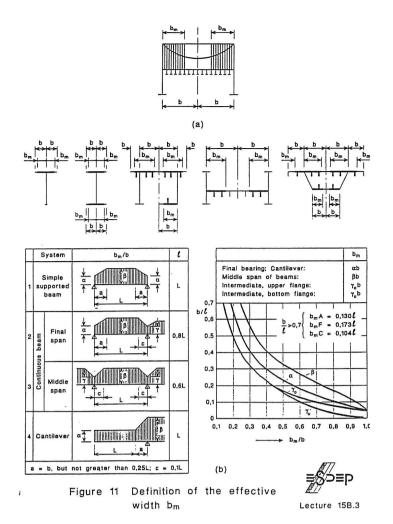


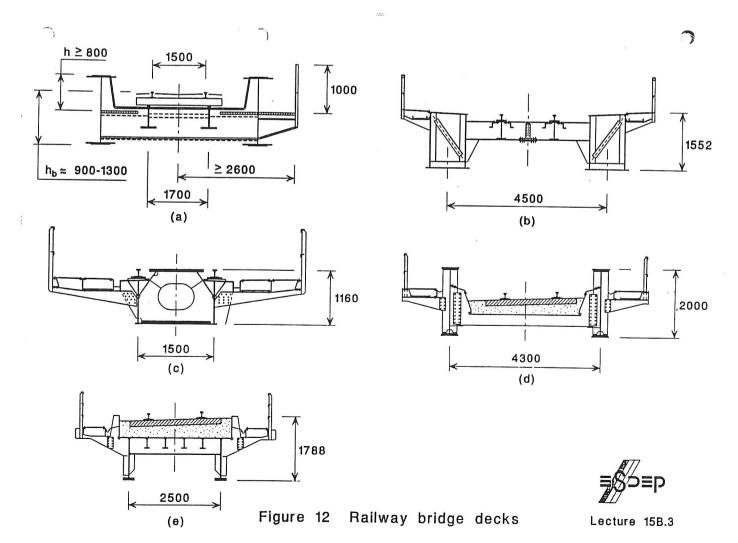
Figure 9 Example of Puchers charts for determining slab moments from local wheel loads.



Figure 10 Standard orthotropic deck.







LECTURE 15B.4

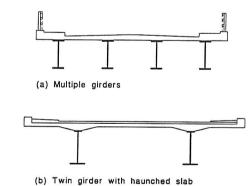
Plate Girder and Beam Bridges

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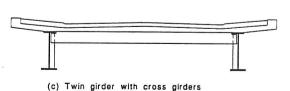
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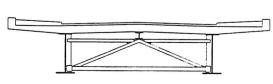
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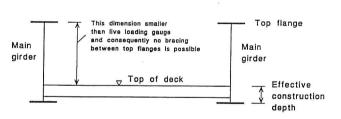
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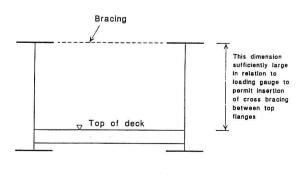


(d) Twin girder with intermediate stringer beam

Figure 1 Types of composite plate girder and girder bridge



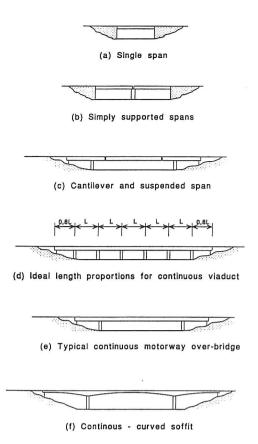
(a) Half-through girder construction



(b) Through girder construction

Figure 2 Half-through and through girder cross-sections.





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Figure 3 Arrangments for longitudinal girders Lecture 15B.4



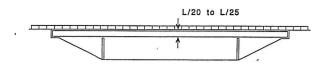
(a) Haunched form (curved soffit)

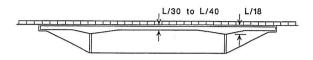


(b) Curved top flange form

Figure 5 Alternative forms of variable depth plate girders







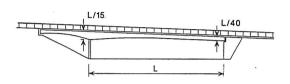
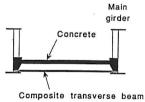


Figure 4 Typical span/depth proportions

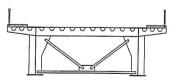


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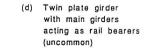


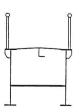
- (a) Half-through arrangement with composite transverse beams
- Half-through arrangement with steel orthotropic deck





(c) Twin plate girder with steel orthotropic deck forming deck-type cross-section







- (e) Twin beam/girder footbridge cross-section
- (f) Half-through arrangement for footbridge

Figure 6 Selection of plate girder/beam bridge configurations



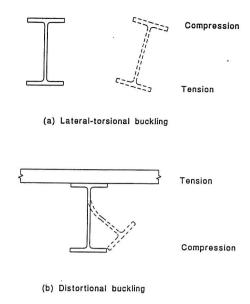
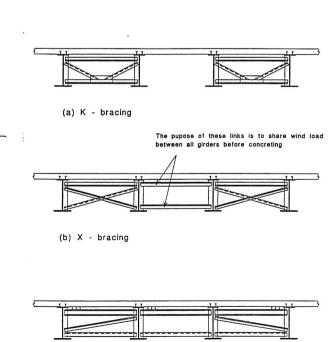


Figure 7 Modes of instability of plate girders



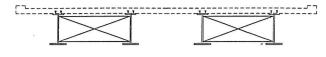


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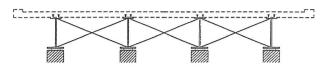
Figure 9 Commonly used bracing systems

(c) Braced abutment trimmer

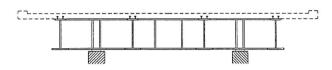




(a) Bracing within spans



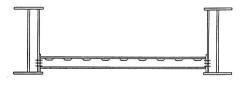
(b) Bracing at abutment and piers



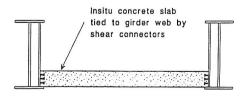
(c) Cross girder at piers

Figure 8 Types of transverse structure for composite plate girder bridges





(a) Discrete U - frame : positive moment



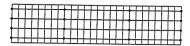
(b) Continuous U - frame : positive moment



(c) Continuous U - frame : negative moment

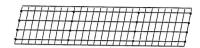
Figure 10 Types of U - frame restraint to compression flange





(a) Orthogonal grillage

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(b) Grillage for spans with small skew (< 20°)



(c) Grillage for spans with large skew (> 20°)

Figure 11 Typical grillages



Lecture 15B.4

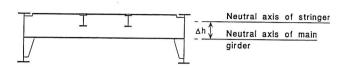
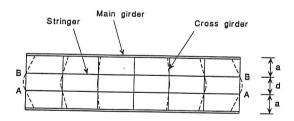


Figure 12 Open grid steel deck difference in neutral axis levels
between stringers and main girders

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Lecture 15B.4



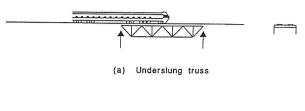
(a) Plan dimensions and deformed shape under uniformly distributed loading



(b) End cross girder deformation

Figure 13 Open grid steel deck - layout and deformations







(b) Semi-through truss (U-frame)

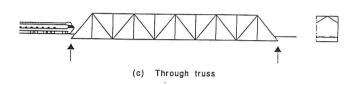
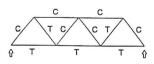
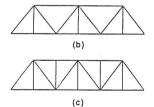


Figure 1 Truss configurations

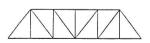




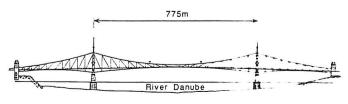
Modern Warren truss: spans 30-150m = Tension C = Compression



(b) and (c) Modified Warren trusses: spans 30-150m: still used for railway bridges



Pratt truss spans 30-100m



Nagy truss in Budapest (1892



(f) Lattice truss interest on y

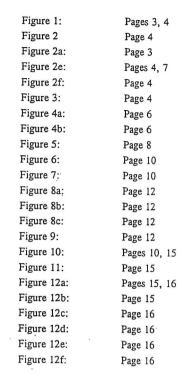
LECTURE 15B.5 Truss Bridges

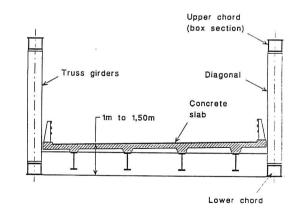
LECTURES

Lecture 1B.6.2:

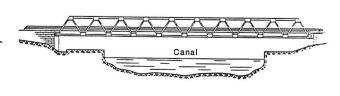
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FIGURES





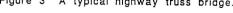
Cross-section

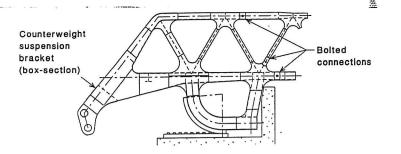


Elevation

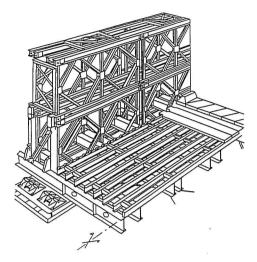
Figure 3 A typical highway truss bridge.







Example of movable truss bridge. Rear part of the bridge.



(b) Bailey temporary bridge for emergency uses.

Particular applications of trusses in bridge construction



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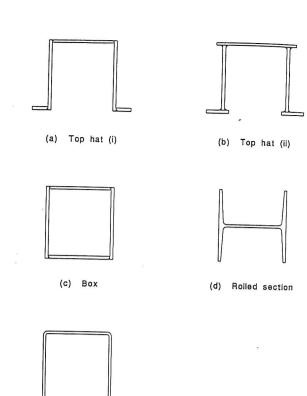


Figure 5 Compression chord members

(f) Rolled hollow section

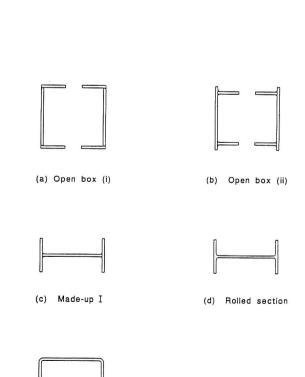
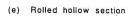


Figure 6 Tension chord members

(a) Open box

(c) Rolled section

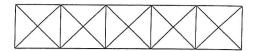




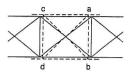
(b) Box

(d) Rolled hollow section

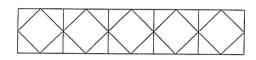
Lecture 15



(a) St.Andrew's cross system

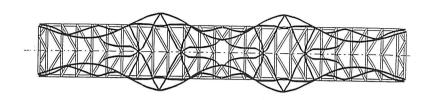


(b) Deformed shape of (a)



(c) Diamond system

Figure 8 Upper chord lateral bracing



Plan: upper chord Note: Truss webs, upper chord and its lateral bracing shown as solid lines for clarity

Figure 9 Buckling mode of a diamond system used as upper chord lateral bracing.



Ecture 15B.

Elevation

Elevation on part span

Section

Plan on top laterals

Plan on bottom laterals

Figure 10 General arrangement of a through truss railway bridge.

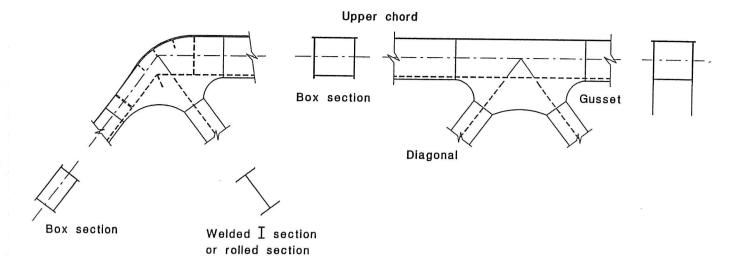
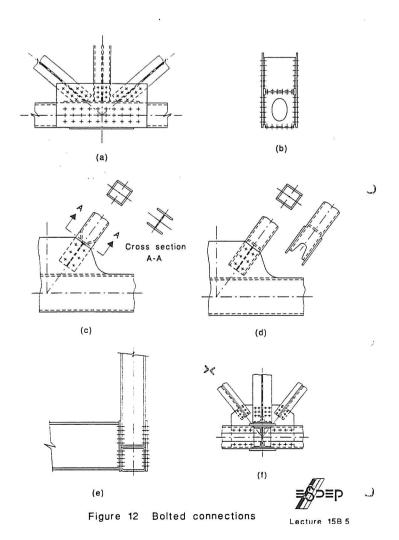


Figure 11 Butt-welded connections and gusset geometries used to avoid fatigue in the connection.





LECTURE 15B.6 Box Girder Bridges

LECTURES

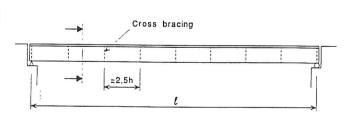
Lecture 8.5.1: Page 10

Lecture 8.5.2: Pages 8 & 10

Lecture 15B.3: Page 8

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Figure 10:	Page 13



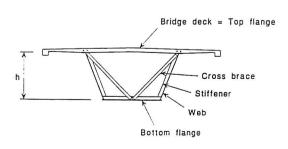


Figure 1 Box girder bridge with composite concrete deck: nomenclature

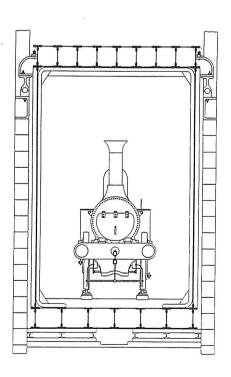
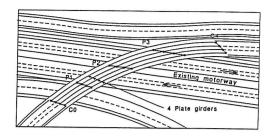


Figure 2 Britannia bridge, 1850.







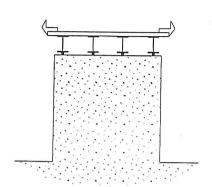
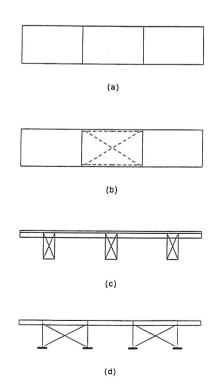


Figure 3a Plate girder bridge.



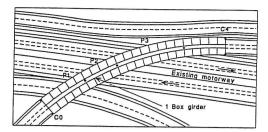
Lecture 15B.6



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Figure 4 Types of cross-section.





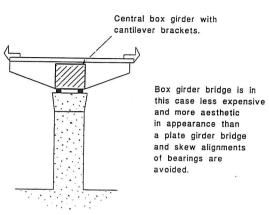
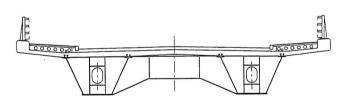


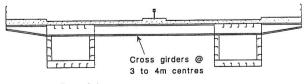
Figure 3b Box girder bridge.



Lecture 15B.6



 (a) Example of diaphragm where distance between boxes is comparable with box width

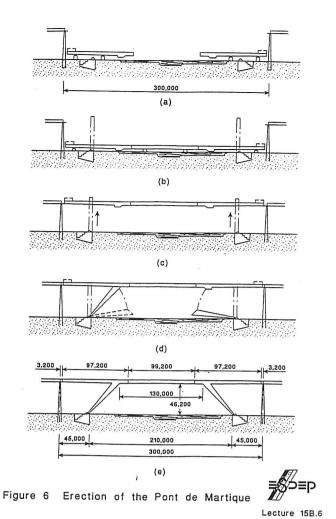


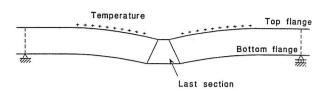
Box girder

(b) Cross girder for widely spaced boxes

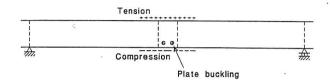
Figure 5 Intermediate transverse elements between boxes.







(a) Closure in presence of temperature differential.



(b) Condition under uniform temperature.

Figure 7 Bridge across Danube, Vienna.



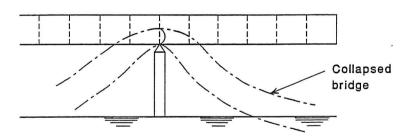


Figure 8 Milford Haven Bridge



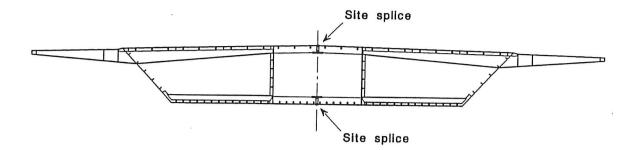


Figure 9 Typical cross-section of Melbourne bridge [2]



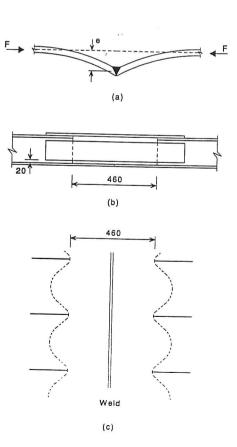


Figure 10 Detail of Koblenz bridge

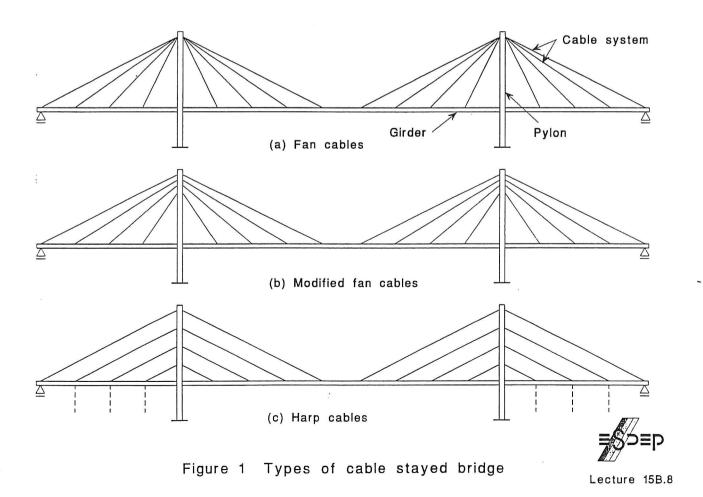


LECTURE 15B.8 Cable-Stayed Bridges

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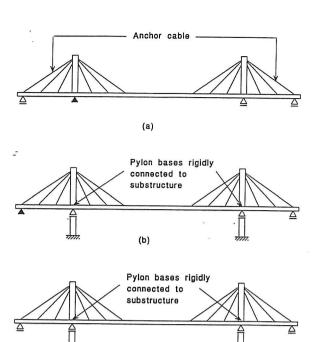


Figure 2 Support conditions for cable stayed bridges

(c)



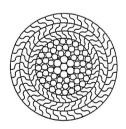


Figure 4 Lock coil cable

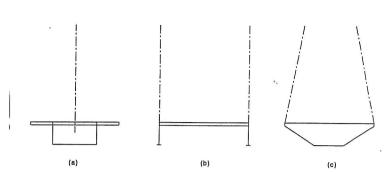


Figure 3 Alternative cable planes



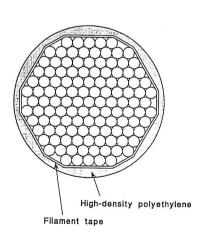


Figure 5 Parallel wire strand (PWS)





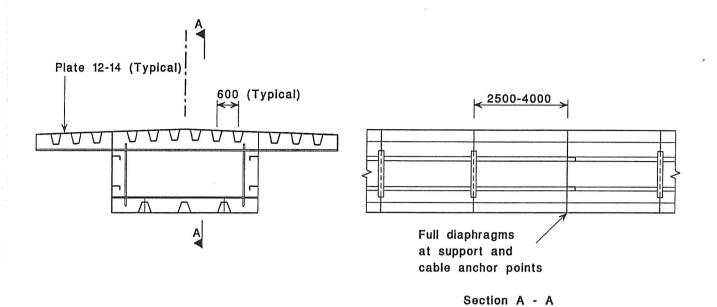


Figure 6 Typical box girder construction

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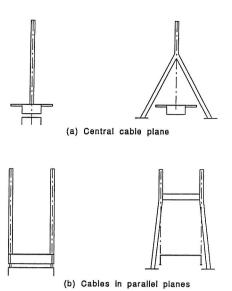


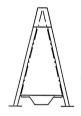
Section A - A

Figure 7 Typical plate girder construction



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(c) Cables in twin inclined planes

Figure 8 Pylon configurations



Lecture 15B.8

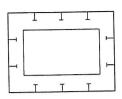
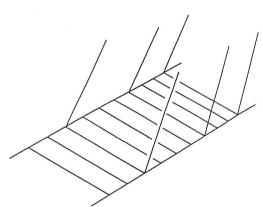
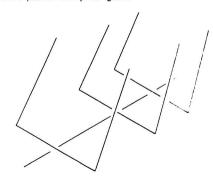


Figure 9 Typical pylon cross-section





(a) Twin cable planes and plate girder



(b) Inclined cable planes and a box girder

Figure 10 Analytical models for cable stayed bridges



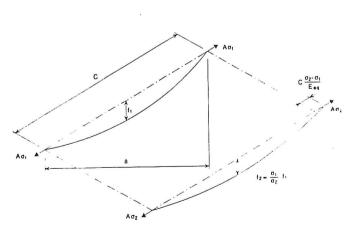


Figure 11 Influence of gravity on cable stiffness



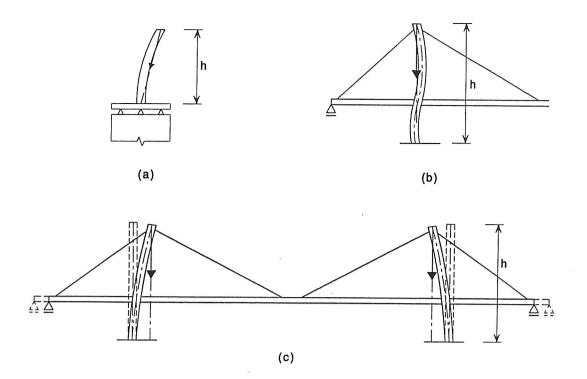


Figure 12 Stability of pylons



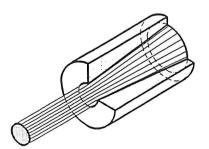


Figure 13 Typical cable anchorage

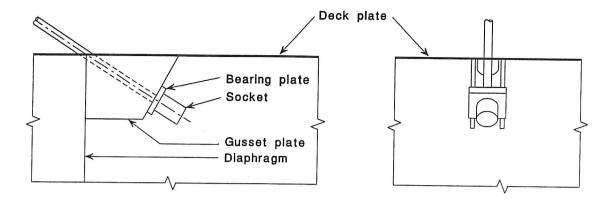


Figure 14 System for anchoring a central cable to a box girder



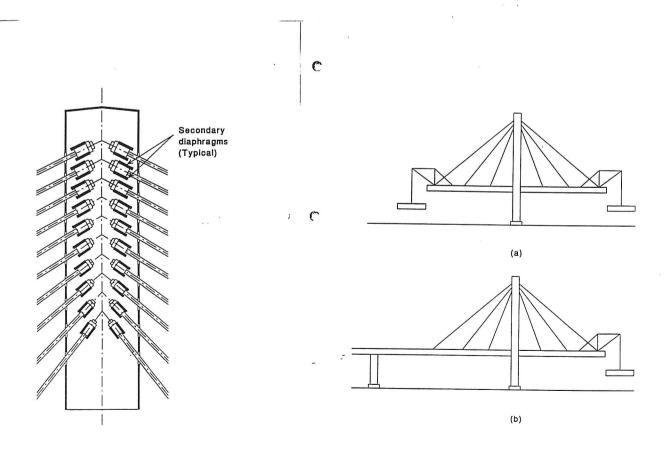


Figure 15 Anchorage system for cables at pylon in a modified fan bridge

Figure 16 Methods of erection





LECTURE 15B.9

Suspension Bridges

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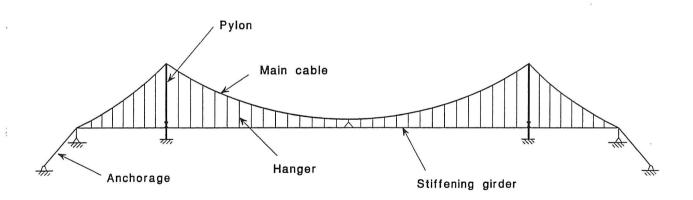


Figure 1 Principal components of a suspension bridge



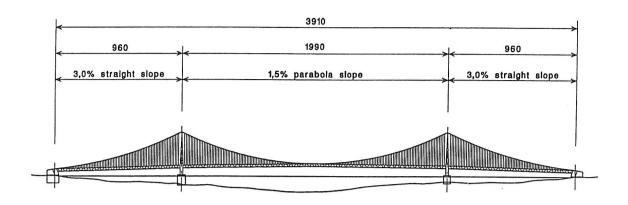
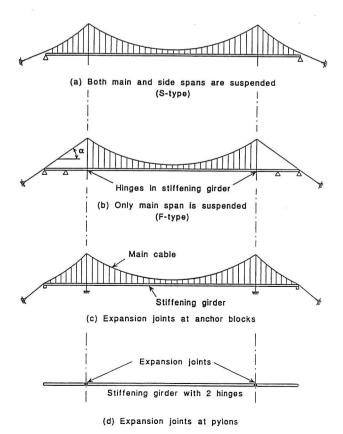


Figure 2 Akashi Kaikyo bridge





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Figure 3 Classification of suspension bridges



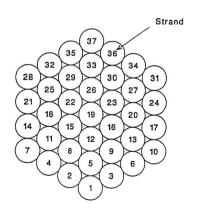


Figure 4 Pattern of strands in cable before compacting



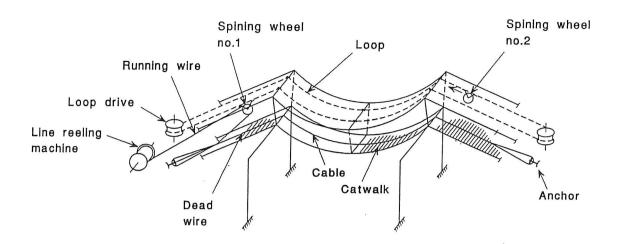
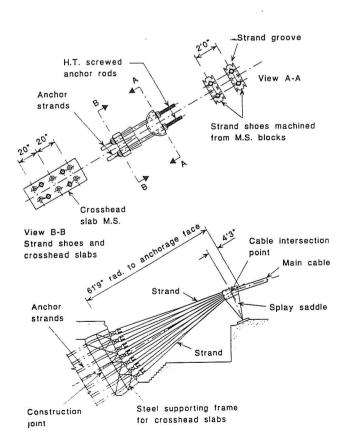
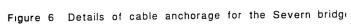


Figure 5 Cable spinning as developed by Roebling

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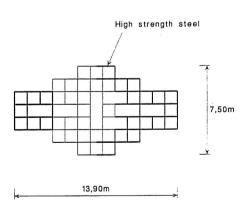


Figure 7 Cross-section of pylon: Golden gate bridge



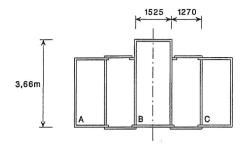


Figure 8 Cross-section of pylon: Firth of Forth bridge

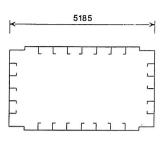


Figure 9 Cross-section of pylon: Severn bridge





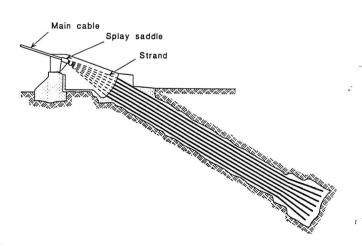


Figure 10 Firth of Forth bridge

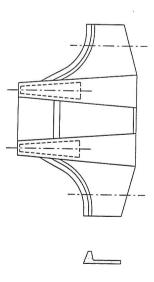
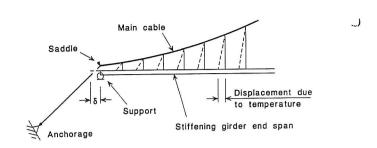


Figure 11 Concrete gravity anchorage:
Lillebelt bridge







Cable

Figure 13 Vortex shedding

Figure 12 Influence of thermal expansion on hangers





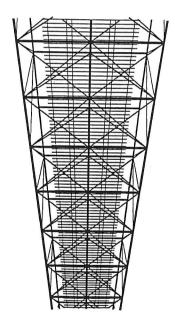


Figure 14 A view from underneath, showing the transparency of the Lisbon suspension bridge



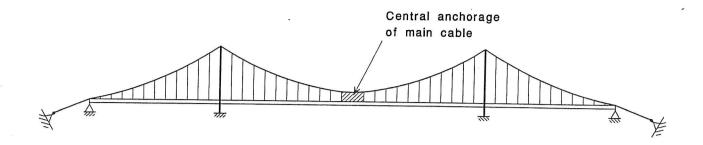


Figure 15 A technique for controlling flutter

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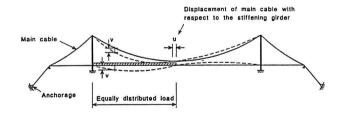
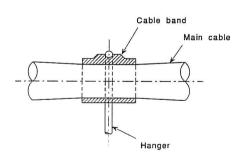


Figure 16 Deformation of the stiffening girder and displacement of the main cable due to a "half span" equally distributed load



igure 17 Cable band to main cable connection



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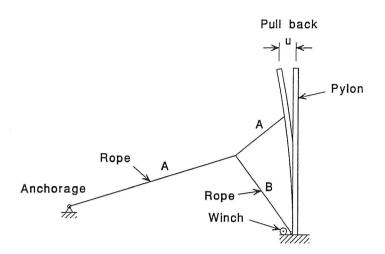


Figure 18 Presetting pylons to prestress anchor portions of main cables

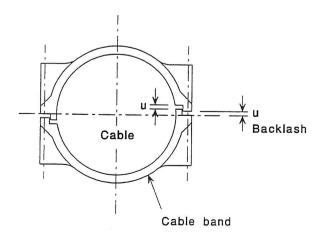


Figure 19 Backlash displacements in cable clamps



Figure 20 Hanger configuration: Severn bridge

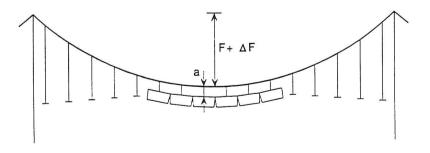


Figure 21 Cable distortion during erection and to influence the on site joints of stiffening girder

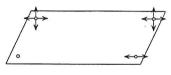
LECTURE 15B.10 Bridge Equipment

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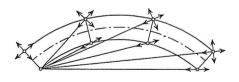
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(a) Simple span.



(b) Long-span viaduct:bearings oriented in direction from the fixed point.



(c) Long-span viaduct:bearings oriented in tangential directions.

Key:

Fixed bearing < ○ >

Unidirectional bearing



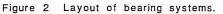






Figure 1 Forces and displacements on bearings

Longitudinal axis

Forces = $V_z H_x H_y$

Displacement: -linear = u_x u_y -rotation = w_x w_y

Transverse

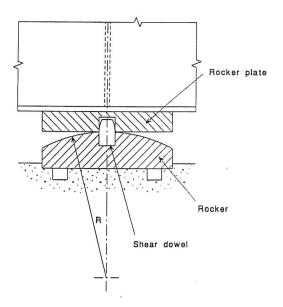


Figure 3 Elevation of line rocker bearing.





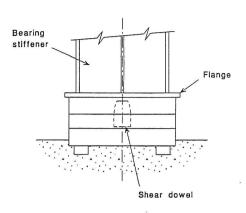


Figure 4 Section through line rocker bearing.



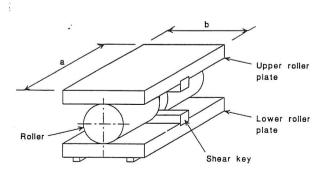


Figure 5 Roller bearing.

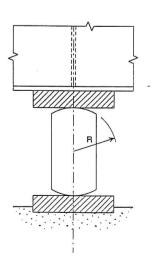


Figure 6 Flat-sided roller bearing.





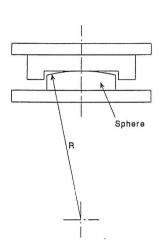


Figure 7 Plane/spherical point bearing.



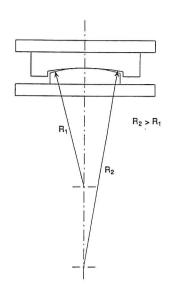


Figure 8 Double spherical point bearing.



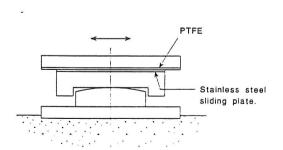


Figure 9 Sliding point bearing.

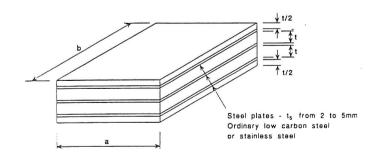


Figure 10 Reinforced elastomeric bearing system





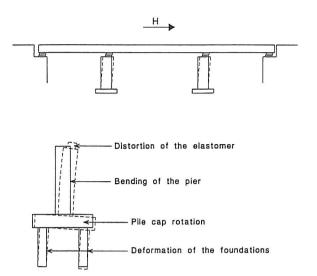


Figure 11 Location and action of the bearings.



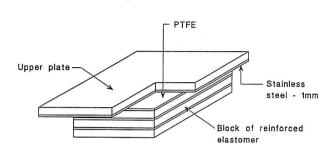


Figure 12 Sliding bearings in reinforced elastomer.



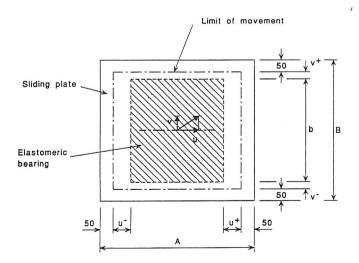


Figure 13 Layout of elastomeric sliding bearing



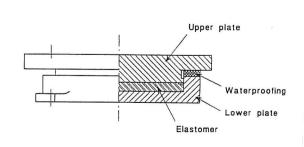


Figure 14 Fixed pot bearing.



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Figure 15 Multidirectional pot bearing.



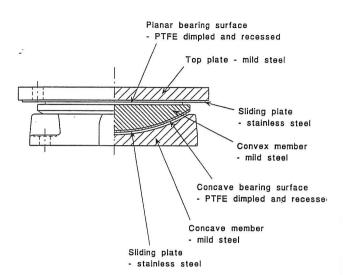


Figure 16 Bearings with spherical gap.



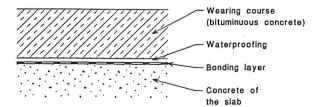


Figure 17 Finishing on concrete slab.

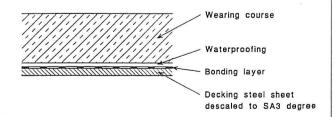
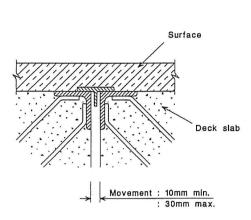


Figure 18 Finishing on orthotropic slab.





(a) Buried joint

Figure 19a Types of covered expansion joint.



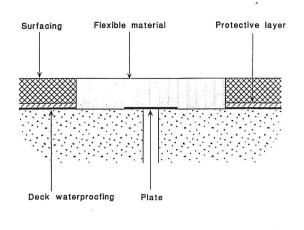
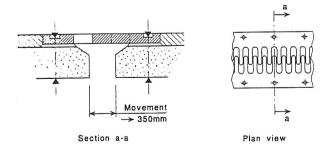


Figure 19b Asphaltic plug joint





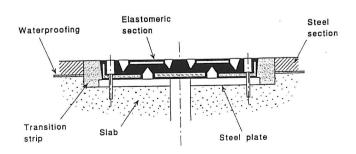
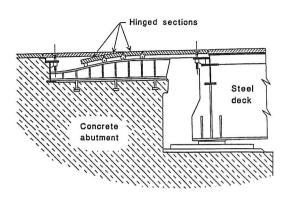


Figure 21 Joint with elastomeric extensions.

Figure 20 Toothed joint.







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Figure 22 Roller shutter joint.

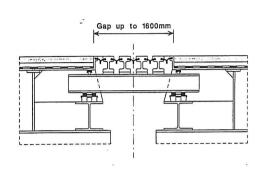
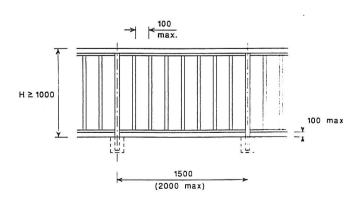


Figure 23 Bellows joints.





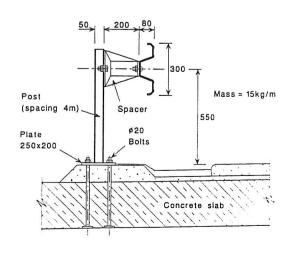


Figure 25 Crash barrier

Figure 24 Pedestrian parapet.





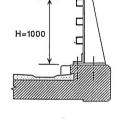
(a) Californian breast high wall

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(b) Barrier in reinforced concrete

Figure 26 Examples of rigid safety fences



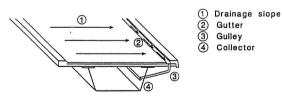
Typical flexible safety fance



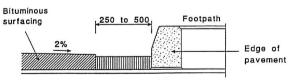
Lecture 15B.10



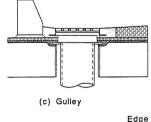
Lecture 15B.

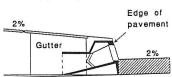


(a) General arrangement



(b) Gutter details

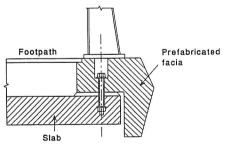




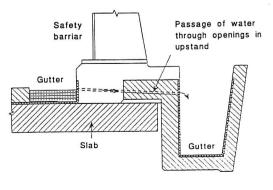
(d) Drainage under pavement

Figure 28 Drainage of rainwater





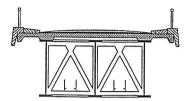
(a) External facia



(b) Facia with integrated gutter

Figure 29 Fascias





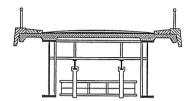


Figure 30 Fixed inspection walkways

Figure 31 Moveable inspection gantry





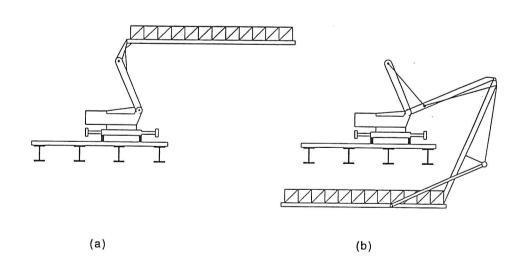


Figure 32 Special mobile inspection equipment



LECTURE 8.5.1 Design of Box Girders

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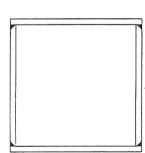


Figure 1 Cross-section of a box girder used in buildings

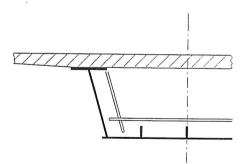


Figure 2 (a) Cross-section of a box beam with composite concrete top flange

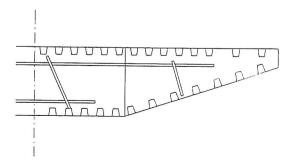
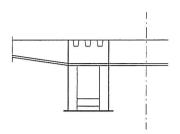


Figure 2 (b) Cross-section of an orthotropic





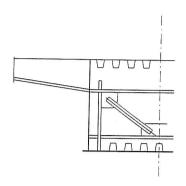


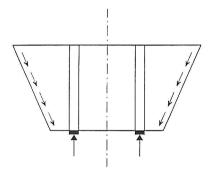
Figure 3 Composite slab with twin box beams

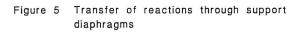
Figure 4 Reinforcement of the cross-section of an orthotropic box beam



Lecture 8.5.1







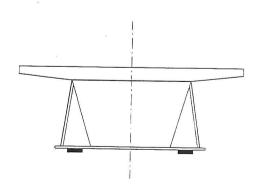
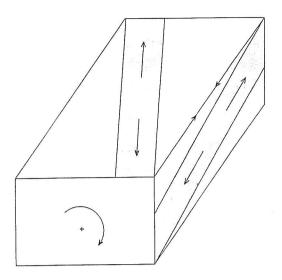


Figure 6 Load-bearing stiffeners external to a box girder







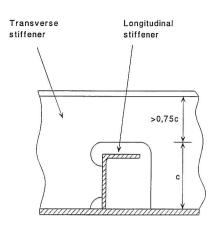
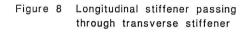


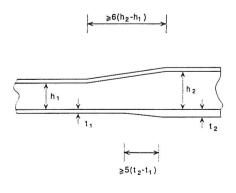
Figure 7 Tension fields developed under torsion





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Lecture 8.5.1



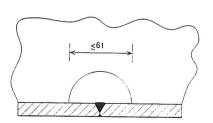


Figure 9 Tapering of longitudinal stiffener

Figure 10 Cut-out to allow butt welding





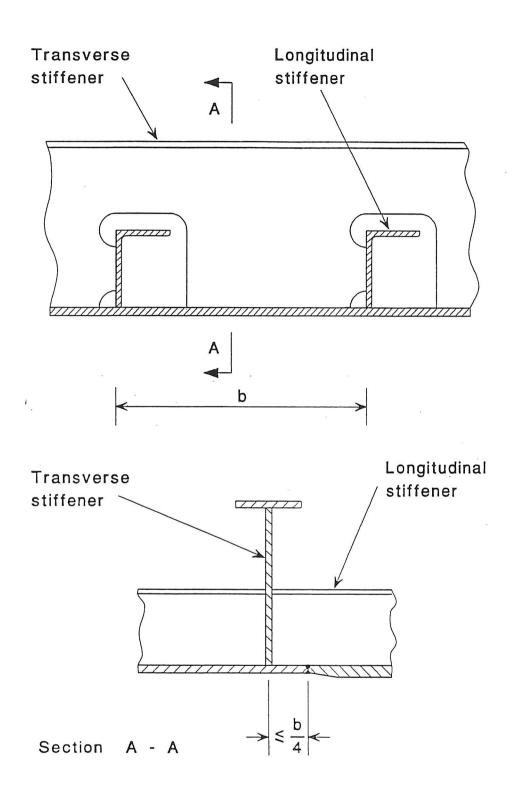


Figure 11 Transverse stiffener welded to thinner plate adjacent to transverse butt weld in the plate

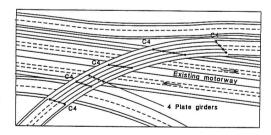
Lecture 8.5.1

LECTURE 8.5.2

Advanced Methods for **Box Girder Bridges**

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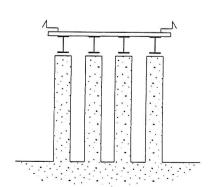
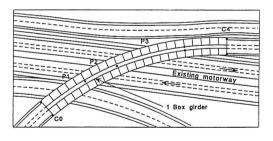


Figure 1 Plate girder bridge





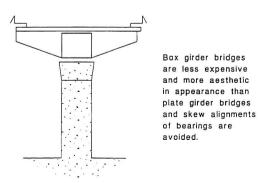
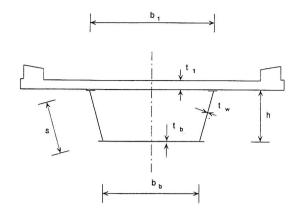


Figure 2 Box girder bridge

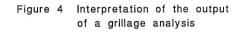




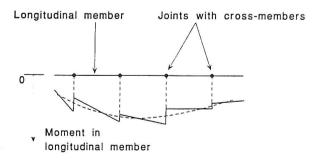
A B C

E F G C

Figure 3 Composite box girder bridge with composite upper deck







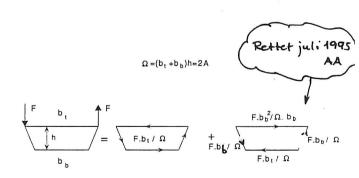


Figure 6 Computation of distortion force

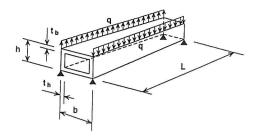
Torsion

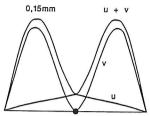
Figure 5 Typical bending or torsional moments given by a grillage analysis





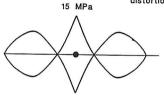
Distortion





Diaphragm t = 5mmu: Top corner deflection due to torsion

> v: Top corner deflection due to distortion



Warping stresses with one single diaphragm

Figure 7A Influence of diaphragm separation - one diaphragm



Lecture 8.5.2

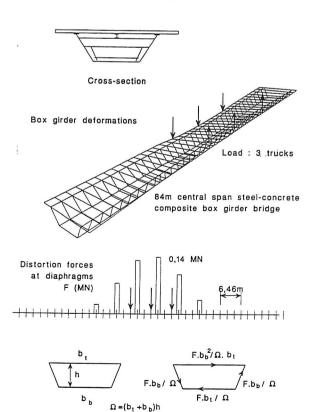


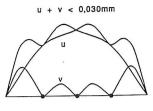
Figure 8 Distribution of the distortion forces between the cross-frames of a bridge

Lecture 8.5.2

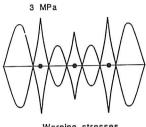
Limit conditions on bearings: flexure rotation-free torsional rotation-blocked undeformable diaphragms at ends no warping stress at the ends

Numerically:

q = 250 N/ml



Diaphrams t = 2mm

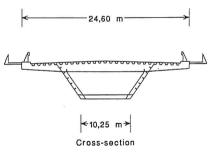


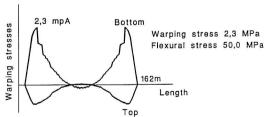
Warping stresses with three diaphragms

Influence of diaphragm separation Figure 7B - three diaphragms

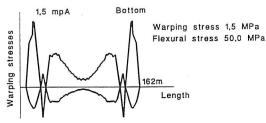


Lecture 8.5.2





Folded plate analysis without additional bracings under uniform traffic torsion



Тор Folded plate analysis with two additional bracings under uniform traffic torsion



Lecture 8.5.2

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