

## **Simulation of the stochastic wave loads using a physical modeling approach**

Liu, W.F.; Sichani, Mahdi Teimouri; Nielsen, Søren R.K.; Peng, Y.B.; Chen, J.B. ; Li, J.

*Published in:*  
Key Engineering Materials

*DOI (link to publication from Publisher):*  
[10.4028/www.scientific.net/KEM.569-570.571](https://doi.org/10.4028/www.scientific.net/KEM.569-570.571)

*Publication date:*  
2013

*Document Version*  
Early version, also known as pre-print

[Link to publication from Aalborg University](#)

*Citation for published version (APA):*  
Liu, W. F., Sichani, M. T., Nielsen, S. R. K., Peng, Y. B., Chen, J. B., & Li, J. (2013). Simulation of the stochastic wave loads using a physical modeling approach. *Key Engineering Materials*, 569-570, 571-578.  
<https://doi.org/10.4028/www.scientific.net/KEM.569-570.571>

### **General rights**

Copyright and moral rights for the publications made accessible in the public portal are retained by the authors and/or other copyright owners and it is a condition of accessing publications that users recognise and abide by the legal requirements associated with these rights.

- Users may download and print one copy of any publication from the public portal for the purpose of private study or research.
- You may not further distribute the material or use it for any profit-making activity or commercial gain
- You may freely distribute the URL identifying the publication in the public portal -

### **Take down policy**

If you believe that this document breaches copyright please contact us at [vbn@aub.aau.dk](mailto:vbn@aub.aau.dk) providing details, and we will remove access to the work immediately and investigate your claim.

# Simulation of the stochastic wave loads using a physical modeling approach

W.F. Liu<sup>1,a</sup>, M.T. Sichani<sup>2,b</sup>, S.R.K. Nielsen<sup>2,c</sup>, Y.B. Peng<sup>3,d</sup>, J.B. Chen<sup>1,e</sup>, and J. Li<sup>1,3,f</sup>

<sup>1</sup>School of Civil Engineering, Tongji University, Shanghai 200092, PR China

<sup>2</sup>Department of Civil Engineering, Aalborg University, 9000 Aalborg, Denmark

<sup>3</sup>State Key Laboratory of Disaster Reduction in Civil Engineering, Tongji University, Shanghai 200092, PR China

<sup>a</sup>2010wfliu@tongji.edu.cn, <sup>b</sup>mts@civil.aau.dk, <sup>c</sup>soren.nielsen@civil.aau.dk,  
<sup>d</sup>pengyongbo@tongji.edu.cn, <sup>e</sup>chenjb@tongji.tu.edu, <sup>f</sup>lijie@tongji.tu.edu

**Keywords:** Wave loads, physical modeling, Monte Carlo, PDEM.

**Abstract.** In analyzing stochastic dynamic systems, analysis of the system uncertainty due to randomness in the loads plays a crucial role. Typically time series of the stochastic loads are simulated using traditional random phase method. This approach combined with fast Fourier transform algorithm makes an efficient way of simulating realizations of the stochastic load processes. However it requires many random variables, i.e. in the order of magnitude of 1000, to be included in the load model. Unfortunately having too many random variables in the problem makes considerable difficulties in analyzing system reliability or its uncertainty. Moreover applicability of the probability density evolution method on engineering problems faces critical difficulties when the system embeds too many random variables. Hence it is useful to devise a method which can make realization of the stochastic load processes with low, say less than 20, number of random variables. In this article we introduce an approach, so-called "physical modeling of stochastic processes", and show its applicability for simulation of the wave surface elevation.

## Introduction

The application of PDEM requires that the stochastic load process and possible uncertain parameter fields related to the structural model are discretized in terms of a small number of basic random variables. With this specific problem in mind Li et al. [1] devised a turbulence model based on so-called evolutionary phases, which are driven merely a single basic random variable. The method generates time varying random phases for the harmonic components of a spectral decomposition of the turbulence process, which are shown to be mutual independent and uniformly distributed in the interval  $[0, 2\pi]$ . Hence, the method is stochastic equivalent to the well-known random phase representation of a turbulence field. In the present paper the same approach has been applied as a stochastic model of the irregular sea-state impinging on the point absorber.

Next we perform some tests that checks similarity of the outputs of this method to that of the standard Monte Carlo simulation. In the first step the statistical properties of the time series generated by this method will be represented which reveals the similarity of this method to the random phase method. Outcomes of the time series simulated by this method will be checked to fulfill the requirements for the Monte Carlo simulation. A truncated double exponential distribution is proposed to be used for the initial evolution time of the phases. Finally, it is shown that the simulated time series generated by this method fulfill the statistical requirements of the process.

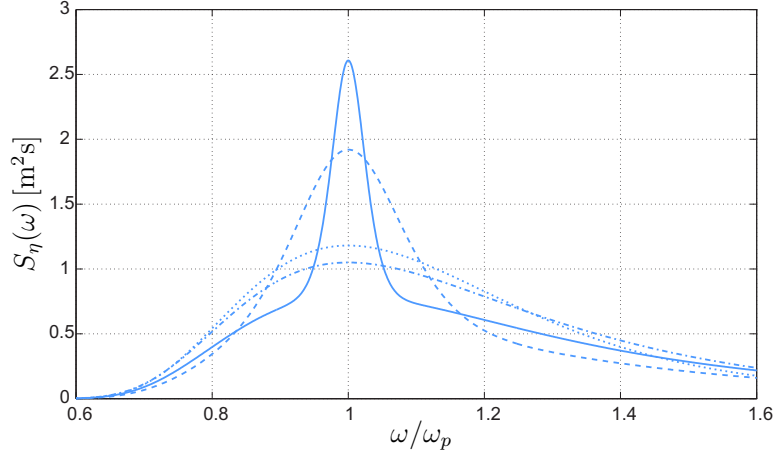


Fig. 1: One-sided modified JONSWAP auto-spectral density function as a function of the bandwidth parameter  $\sigma_f$ ,  $H_s = 3\text{m}$ . —:  $\sigma_f=0.03$ . - - -:  $\sigma_f=0.1$ . . . . :  $\sigma_f=0.5$ . - · - · - :  $\sigma_f=1.0$ .

### Stochastic Wave Load Model

The irregular plane waves are assumed to propagate in the positive  $x$ -direction. The surface elevation at the position  $x$  at the time  $t$  is described by the homogeneous and stationary zero mean Gaussian process  $\{\eta(x, t), (x, t) \in R^2\}$ . The double sided auto-spectral density function of the sea-surface elevation process at a given position is defined by the following slightly modified double-sided version of the JONSWAP spectrum, (Hasselmann *et al.* 1973)

$$S_{\eta\eta}(\omega) = \beta \frac{H_s^2}{\omega_p} \gamma^\alpha \left( \frac{|\omega|}{\omega_p} \right)^{-5} \exp\left(-\frac{5}{4} \left( \frac{\omega}{\omega_p} \right)^{-4}\right) \quad (1)$$

where

$$\left. \begin{aligned} \alpha &= \exp\left(-\frac{1}{2} \left( \frac{|\omega| - \omega_p}{\sigma_f \omega_p} \right)^2\right) \\ T_p &= \frac{2\pi}{\omega_p} = \sqrt{\frac{180 H_s}{g}} \\ \gamma &= 3.3 \end{aligned} \right\} \quad (2)$$

$T_p$  is the peak period,  $\omega_p = 2\pi/T_p$  is the related angular peak frequency,  $H_s$  is the significant wave height, and  $\sigma_f$  is a band-width parameter.  $\beta$  is a normalization parameter to be determined, so the relation  $H_s = 4.0\sigma_\eta$  is fulfilled, corresponding to Rayleigh distributed wave heights. The auto-spectral density function has been shown in Figure 1 for various values of  $\sigma_f$ . Notice the abscissa has been normalized with respect to the angular peak frequency  $\omega_p$ , which depends on  $H_s$  as indicated in the second relation in (2). The surface elevation field admits the following spectral representation

$$\eta(x, t) = \sum_{j=1}^J \sqrt{2} \eta_j \cos(\omega_j t - k_j x - \Phi_j) \quad , \quad \omega_j = (j-1)\Delta\omega \quad (3)$$

$$\eta_j = \sqrt{2S_{\eta\eta}(\omega_j) \Delta\omega} \quad (4)$$

where  $J$  is the number of harmonic components in the spectral decomposition,  $\eta_j$  denotes the standard deviation of harmonic components with angular frequencies in the interval  $[\omega_j, \omega_j + \Delta\omega]$ , and  $\Phi_j$  are

mutually independent identical distributed random variables, uniformly distributed in  $[0, 2\pi]$ .  $k_j$  is the wave number of the  $j$ th harmonic wave component. This is related to the angular frequency  $\omega_j$  by the well-known dispersion relation of linear gravity waves, (Svendsen and Jonsson 1980)

$$k_j h \tanh(k_j h) = \frac{\omega_j^2 h}{g} \quad (5)$$

The phases  $\Phi_j$  are referred to as the basic variables of the model. The number  $J$  of these variables is typically  $10^2 - 10^4$ , which renders the model (28) useless for application in the PDEM. Inspired by the so-called evolutionary phase approach by Li *et al.*, (Li *et al.* 2012b) for handling the analog problem of one-dimensional frozen turbulence processes propagating non-dispersive with the mean wind velocity according to Taylor's hypothesis of frozen turbulence, the following alternative spectral representation is suggested

$$\bar{\eta}(x, t) = \sum_{j=1}^J \sqrt{2} \eta_j \cos(\omega_j t - k_j x - \bar{\Phi}_j) \quad (6)$$

where

$$\bar{\Phi}_j = \text{mod}(k_j \omega_j \eta_j T_0, 2\pi) \quad (7)$$

$k_j \omega_j \eta_j T_0$  is referred to as the evolutionary phases, and  $\vartheta_j = k_j \omega_j \eta_j$  denotes a characteristic velocity of the fluid particles in the wave motion.  $T_0$  is an assumed common origin of time for the evolutionary phases, which is considered a random variable. Since the auto-spectral density function is determined by the amplitude of the harmonic components, both representations (3) and (6) represent the exact auto-spectral density  $S_{\eta\eta}(\omega)$  for arbitrary values of the phases  $\Phi_j$  and  $\bar{\Phi}_j$  within an error determined by the discretization of the spectrum as given by the parameter  $\Delta\omega$ . The important point is that the latter representation is generated by merely a single random variable.

### Probability distribution of $T_0$

The conventional way to obtain the Probability Density Function (PDF) of the  $T_0$  is to measure time series of the wave surface elevation and perform a backward analysis on each one of them to obtain its  $T_0$ . Finally an empirical PDF can be fitted to these calculated values. This is performed on the data obtained from the buoys installed in Hanstholm, Denmark with the procedure described in the following. The basic procedure is described in [2]. The basic phase of harmonic wave in the peak frequency is defined as  $\Phi(\omega_p, 0)$ , and its phase evolutionary velocity of the fluid particle is defined as  $\vartheta_p = k_p \omega_p \eta_p$ . Propagating the wave inversely from the commencement of time history, a series of values of time quantity  $T_0$ , whereby the phase value  $\Phi(\omega_p, 0) = 2\pi(n - 1)$ , can be derived from the following equation

$$T_0 = \frac{\Phi(\omega_p, 0)}{k_p \omega_p \eta_p} + \frac{2\pi(n - 1)}{k_p \omega_p \eta_p}, \quad n = 1, \dots, N_u \quad (8)$$

Where  $T_0$  is a time quantity with positive values;  $n$  denotes the number domain of starting-time that belongs to natural numbers;  $N_u$  is limited to be  $10^9$  due to computational costs. The phase value at a typical  $T_0$  is solved by

$$\Phi(\omega_p, T_0) = \Phi(\omega_p, 0) - k_p \omega_p \eta_p T_0, \quad j = 1, \dots, m \quad (9)$$

If  $T_0$  satisfies the following constraint condition of maximum deviation

$$| \text{mod} \{ \Phi(\omega_p, T_0), 2\pi \} | \leq \epsilon \quad (10)$$

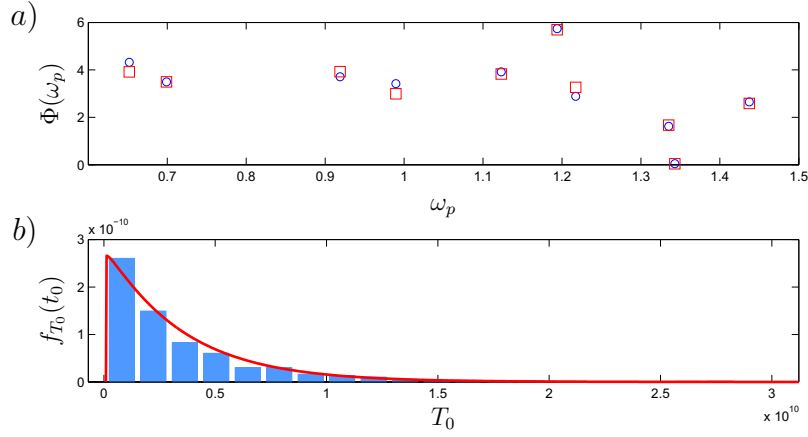


Fig. 2: Analysis of the phases of the wave elevations; a) Measured ( $\circ$ ) versus fitted ( $\square$ ) phases of the time series, b) Histogram and fitted PDF of the measured  $T_0$ s.

where  $\epsilon$  is the error tolerance. The corresponding  $T_0$  is then viewed as the desirable starting-time of phase evolution. Since the limited lower frequency harmonic waves contain the primary information and most energy of turbulence, identification of the starting-time of phase evolution is only focused on the lower frequency domain. In general it is extremely difficult to derive the starting-time in every frequency point. Hence the phases related to only a limited number of harmonics within the low frequency range, where most of the energy of the process is concentrated, are used to obtain  $T_0$ . Here only 10 phases at high energy part of the surface wave spectrum are used for this purpose.  $T_0$  of totally 2500 measured time series of 800-sec wave elevation (from Hanstholm), whose significant wave height is about 3m, were identified first. The proposed truncated double exponential distribution is then fitted to the identified  $T_0$ s. The quality of the fit is shown in figure

### Statistical properties of the simulated surface elevations

Three values for  $T_0$  are then chosen from the three regions obtained by the previous analysis and time series of the surface elevation associated to them are simulated. The resulted realizations are shown in figure 3. It is clear that both for very small and very large values of  $t_0$  the realizations of the wave surface elevation deviates from the way they should look like. Only when  $t_0$  is chosen in the calculated range the realization seems to be correct. The histogram of the generated phases for the same  $t_0$  values used in the previous analysis are shown in figure 4. Again it can be seen that distribution of the phases shown in figures 4.a and 4.c deviate to some extent from the uniform distribution while figure 4.b fits better than the former two to the uniform distribution. Here, a truncated exponential distribution (11) is suggested. The scale parameter  $t_3$  specifies the mean value (and variance) of  $T_0$ . In appendix A it is shown that the distribution of the phases which their  $T_0$ 's are drawn from this family of distributions asymptotically approaches uniform distribution.

$$f_{T_0}(t) = \begin{cases} \frac{\exp\left(-\frac{t-t_1}{t_3}\right)}{t_3 \left(1 - \exp\left(-\frac{t_2-t_1}{t_3}\right)\right)} & , \quad t \in [t_1, t_2] \\ 0 & , \quad t \notin [t_1, t_2] \end{cases} \quad (11)$$

Next we try to check the distribution of the samples of the generated surface elevations using Monte Carlo simulation. Here we have used 5000 samples with different randomly chosen  $t_0$ 's in the defined range. It is expected that the generated realizations of the wave surface elevation are Gaussian distributed in every time instant. It is seen that the estimated histogram fits acceptably to the normal distribution with the analytical variance of the wave elevation  $\sigma_\eta^2 = H_s^2/4$ . Moreover since the process

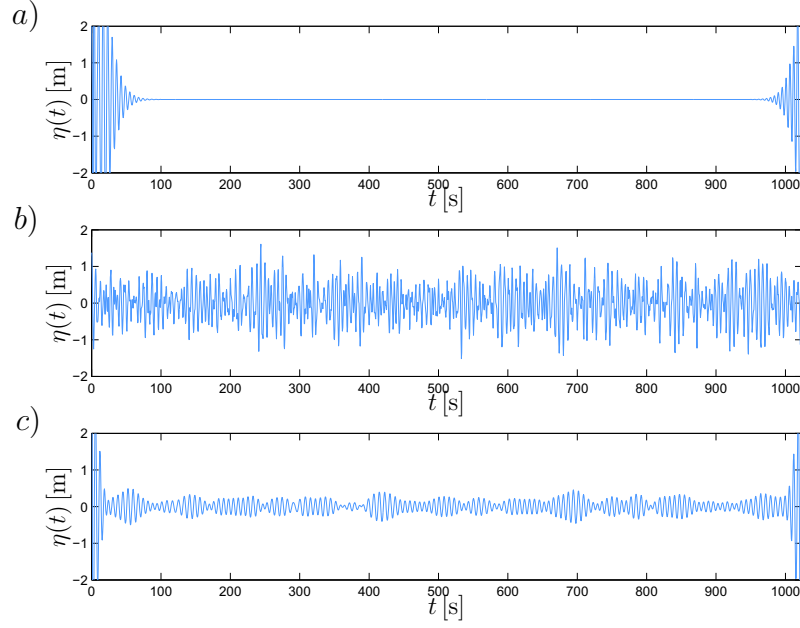


Fig. 3: Samples of the wave elevation time series for different values of  $t_0$ ; a)  $t_0 = 10^3$  b)  $t_0 = 10^{10}$  c)  $t_0 = 5 \times 10^{18}$ .

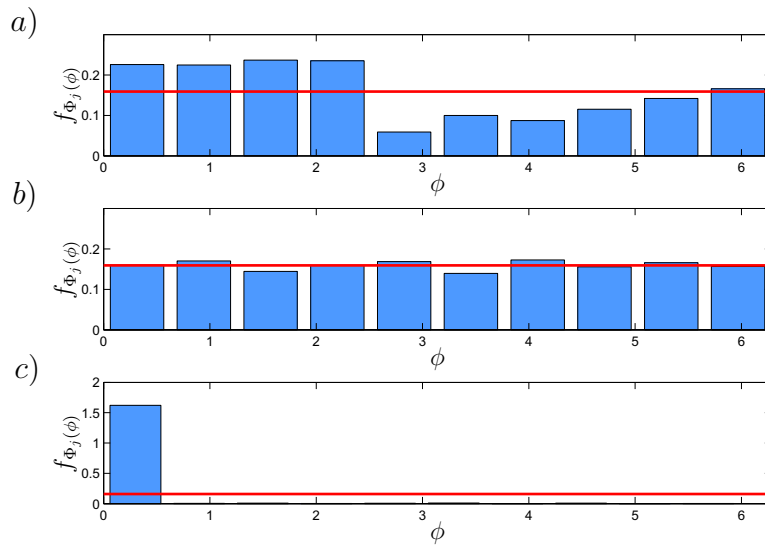


Fig. 4: Histogram of sampled phases  $\bar{\Phi}_j$ ,  $H_s = 3$  m,  $\sigma_f = 0.1$ ,  $J=2048$ ; a)  $t_0 = 10^3$  s, b)  $t_0 = 10^{10}$  s, c)  $t_0 = 5 \times 10^{18}$  s.

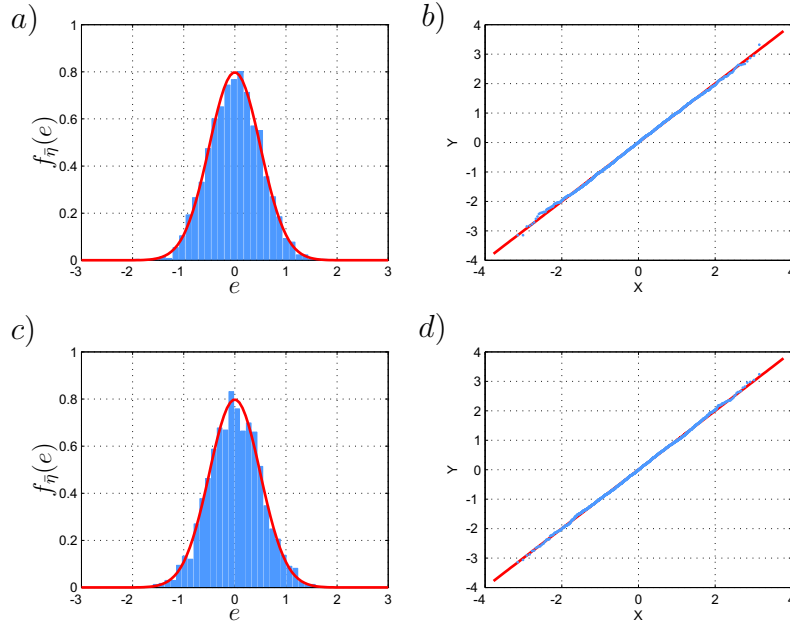


Fig. 5: Statistical properties of the simulated time series by the proposed empirical PDF; a) PDF of the surface elevation  $\eta(t)$  obtained as ensemble average of one realization, b) Comparison of histogram of samples of one realization with Gaussian distribution  $\mathcal{N} \sim (0, \sigma_\eta^2)$ , c) PDF of the surface elevation  $\eta(t)$  obtained as ensemble average at the time  $t = 732$  s of 5000 time-series, d) Comparison of histogram with Gaussian distribution  $\mathcal{N} \sim (0, \sigma_\eta^2)$ .

is stationary, the distribution of the samples could be achieved by ergodic sampling. Figures 5.a and 5.b show the estimated PDF and QQ-plots of samples of one realization of wave surface elevation by ergodic sampling respectively. Figures 5.c and 5.d show the estimated PDF and QQ-plots of the samples of 5000 realization of wave surface elevation at a certain time using Monte Carlo simulation respectively. Finally the covariance function of the samples generated by the introduced method are shown in figure 6. The covariance of the samples of the process are estimated as ensemble average of 100 samples of the process simulated by the introduced method. For both too small and too large values of  $t_0$  the covariances in the non zero time lags underestimated the covariance of the process. All figures confirm that distribution of the samples is well matched with that of Gaussian. As verified by these results the  $\eta(x, t)$  becomes zero-mean and normal distributed, if the sample values of  $T_0$  is drawn from a finite interval  $[t_1, t_2]$ . Under this condition (6) becomes stochastically equivalent to (3).

## Concluding Remarks

A new method for modeling wave surface elevation is represented in the article. The concept of the method is similar to that of the random phase. The advantage of the method is that it binds all the random phases of the problem into one by some technique. It is useful in view of applying PDEM on stochastic dynamic systems where appreciably high number of random variables are associated in analysis. Normally most of these random numbers come from the stochastic loads of the model i.e. wave loads or seismic loads. Dynamic analysis of structural systems excited by such loads involves thousands of random variables which is out of reach of PDEM. Hence such a method which can decrease the number of the random variables appreciably is necessary to be used in this regards. It is shown in the paper that this method fulfills the statistical requirements of the stochastic process i.e. uniform distribution of random phases and Gaussian distribution of the samples of the process and correct power spectrum.

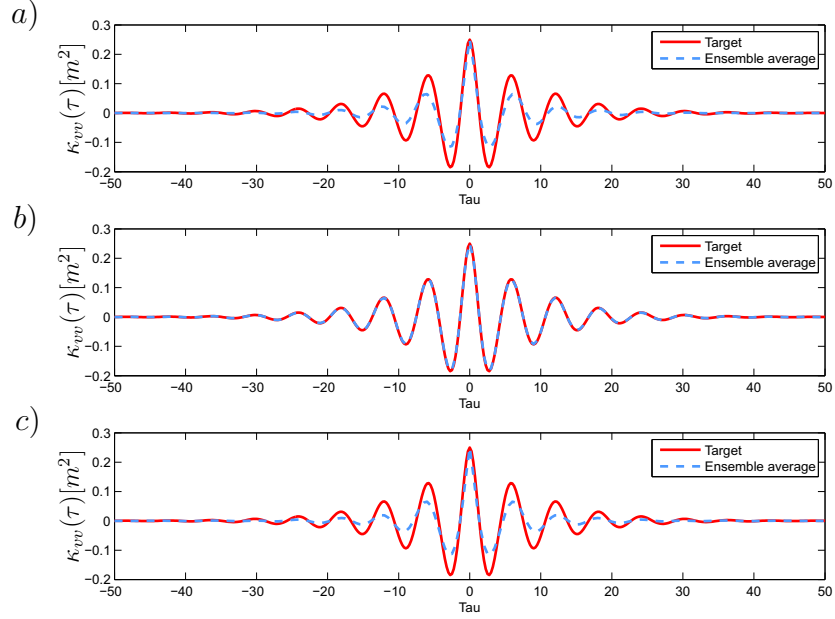


Fig. 6: Auto covariance function of the simulated time series; a)  $1 < t_0 < 10^2$  b)  $10^4 < t_0 < 10^6$  c)  $10^{18} < t_0 < 10^{20}$ .

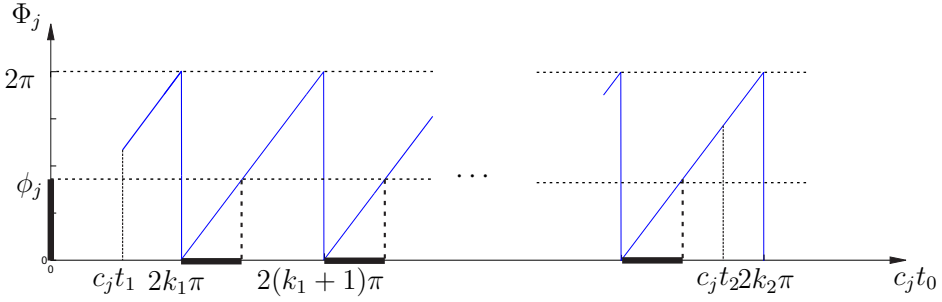


Fig. 7: Schematic representation of the phases generated by the physical modeling method.

## Acknowledgements

The authors gratefully acknowledge the financial support from the Danish Council for Strategic Research under the Programme Commission on Sustainable Energy and Environment (Contract 09-067257, Structural Design of Wave Energy Devices). The support from the National Natural Science Foundation of China (NSFC) (Grant No.51250110538) is gratefully acknowledged by the authors.

## Appendix

Distribution of the random phases generated by the proposed method is committed here. Assume  $t_0$  is distributed as

$$f_{T_0}(t_0) = \frac{1}{t_3\beta} e^{-\frac{t_0-t_1}{t_3}} \quad , \quad \beta = 1 - e^{-\frac{t_2-t_1}{t_3}} \quad (12)$$

according to figure 7 we have

$$\left. \begin{aligned} F_{\Phi_j}(\phi_j) &= p(\Phi_j < \phi_j) = \sum_{k=k_1}^{k_2} p(2k\pi < c_j t_0 < 2k\pi + \phi_j) \\ &= \sum_{k=k_1}^{k_2} \left( F_{T_0}\left(\frac{2k\pi + \phi_j}{c_j}\right) - F_{T_0}\left(\frac{2k\pi}{c_j}\right) \right) \end{aligned} \right\} \quad (13)$$



where  $c_j t_1 = 2(k_1 - 1)\pi + \text{mod}(c_j t_1, 2\pi)$  and  $c_j t_2 = 2(k_2 - 1)\pi + \text{mod}(c_j t_2, 2\pi)$ . Then

$$f_{\Phi_j}(\phi_j) = \frac{d}{d\phi_j} F_{\Phi_j}(\phi_j) = \frac{1}{c_j} \sum_{k=k_1}^{k_2} f_{T_0} \left( \frac{2k\pi + \phi_j}{c_j} \right) \quad (14)$$

Next, the characteristic function of  $\Phi_j$  can be calculated [3]

$$S_{\Phi_j}(\omega) = \int_0^{2\pi} e^{i\omega\phi_j} f_{\Phi_j}(\phi_j) d\phi_j = \int_0^{2\pi} \frac{e^{i\omega\phi_j}}{c_j} \sum_{k=k_1}^{k_2} f_{T_0} \left( \frac{2k\pi + \phi_j}{c_j} \right) d\phi_j \quad (15)$$

$$= \sum_{k=k_1}^{k_2} \frac{1}{c_j} \int_0^{2\pi} e^{i\omega\phi_j} f_{T_0} \left( \frac{2k\pi + \phi_j}{c_j} \right) d\phi_j$$

substituting (12) into (15) results

$$S_{\Phi_j}(\omega) = \sum_{k=k_1}^{k_2} \frac{1}{c_j t_3 \beta} \int_0^{2\pi} \exp \left( i\omega\phi_j - \frac{2k\pi + \phi_j - c_j t_1}{c_j t_3} \right) d\phi_j \quad (16)$$

$$= \left( \frac{e^{t_1/t_3}}{c_j t_3 \beta} \int_0^{2\pi} e^{\gamma\phi_j} d\phi_j \right) \times \sum_{k=k_1}^{k_2} \left( e^{-\frac{2\pi}{c_j t_3}} \right)^k$$

$$= \left( \frac{e^{t_1/t_3}}{c_j t_3 \beta \gamma} (e^{2\pi\gamma} - 1) \right) \times \sum_{k=k_1}^{k_2} \left( e^{-\frac{2\pi}{c_j t_3}} \right)^k = \frac{e^{t_1/t_3}}{c_j t_3 \beta \gamma} \frac{\Omega^{k_1} - \Omega^{k_2+1}}{1 - \Omega} (e^{2\pi\gamma} - 1)$$

where  $\gamma = i\omega - \frac{1}{c_j t_3}$  and  $\Omega = \exp(-\frac{2\pi}{c_j t_3})$ . As a special case assume  $t_1 \rightarrow 0$  and  $t_2 \rightarrow \infty$  then  $k_1 \rightarrow 0, k_2 \rightarrow \infty$  and  $\beta \rightarrow 1$ . For this case we have

$$f_{T_0}(t_0) = \frac{1}{t_3} e^{-\frac{t_0}{t_3}} \Leftrightarrow S_{\Phi_j}(\omega) = \frac{1}{c_j t_3 \gamma} \frac{e^{2\pi\gamma} - 1}{1 - \Omega} \quad (17)$$

As  $c_j t_3 \rightarrow \infty$

$$\lim_{c_j t_3 \rightarrow \infty} S_{\Phi_j}(\omega) = \frac{e^{i2\pi\omega} - 1}{i2\pi\omega} \quad (18)$$

(18) proves that the  $T_0$  will be distributed uniformly between 0 and  $2\pi$  asymptotically as  $c_j t_3 \rightarrow \infty$  for the proposed distribution.

## References

- [1] J. Li, J. Chen, Stochastic Dynamics of Structures, 1st Edition, Wiley, 2009.
- [2] J. Li, Y. Peng, Q. Yan, Modeling and simulation of fluctuating wind speeds using evolutionary phase spectrum, Probabilistic Engineering Mechanics (0) (2013) -. doi:10.1016/j.probengmech.2013.01.001.
- [3] Y. K. Lin, G. Cai, Probabilistic Structural Dynamics, McGraw-Hill, 2004.