

Modal Analysis of an Offshore Platform Using Two Different ARMA Approaches

Brincker, Rune; Andersen, P.; Martinez, M. E. ; Tallavó, F.

Publication date:
1995

Document Version
Early version, also known as pre-print

[Link to publication from Aalborg University](#)

Citation for published version (APA):

Brincker, R., Andersen, P., Martinez, M. E., & Tallavó, F. (1995). *Modal Analysis of an Offshore Platform Using Two Different ARMA Approaches*. Dept. of Building Technology and Structural Engineering, Aalborg University. Fracture and Dynamics Vol. R9531 No. 66

General rights

Copyright and moral rights for the publications made accessible in the public portal are retained by the authors and/or other copyright owners and it is a condition of accessing publications that users recognise and abide by the legal requirements associated with these rights.

- Users may download and print one copy of any publication from the public portal for the purpose of private study or research.
- You may not further distribute the material or use it for any profit-making activity or commercial gain
- You may freely distribute the URL identifying the publication in the public portal -

Take down policy

If you believe that this document breaches copyright please contact us at vbn@aub.aau.dk providing details, and we will remove access to the work immediately and investigate your claim.

FRACTURE & DYNAMICS
PAPER NO. 66

To be presented at the 14th International Modal Analysis Conference
Dearborn, Michigan, USA, February 12-15, 1996

R. BRINCKER, P. ANDERSEN, M. E. MARTINEZ, F. TALLAVÓ
MODAL ANALYSIS OF AN OFFSHORE PLATFORM USING TWO DIFFERENT ARMA APPROACHES
NOVEMBER 1995

ISSN 1395-7953 R9531

The FRACTURE AND DYNAMICS papers are issued for early dissemination of research results from the Structural Fracture and Dynamics Group at the Department of Building Technology and Structural Engineering, University of Aalborg. These papers are generally submitted to scientific meetings, conferences or journals and should therefore not be widely distributed. Whenever possible reference should be given to the final publications (proceedings, journals, etc.) and not to the Fracture and Dynamics papers.

FRACTURE & DYNAMICS
PAPER NO. 66

To be presented at the 14th International Modal Analysis Conference
Dearborn, Michigan, USA, February 12-15, 1996

R. BRINCKER, P. ANDERSEN, M. E. MARTINEZ, F. TALLAVÓ
MODAL ANALYSIS OF AN OFFSHORE PLATFORM USING TWO DIFFERENT ARMA APPROACHES
NOVEMBER 1995

ISSN 1395-7953 R9531

Modal Analysis of an Offshore Platform Using Two Different ARMA Approaches

R. Brincker & P. Andersen

Department of Building Technology and Structural Engineering
Aalborg University, Denmark

M.E. Martinez & F. Tallavó

INTEVEP, S.A.
Research and Technology Center for Petróleos de Venezuela

Abstract

In the present investigation, multi-channel response measurements on an offshore platform subjected to wave loads is analysed using Auto Regressive Moving Average (ARMA) models. Two different estimation schemes are used and the results are compared. In the first approach, a scalar ARMA model is used to estimate the poles (eigenfrequencies and damping ratios), and then the mode shapes are found by fitting an analytical form to the empirical estimates of the covariance functions using the estimated poles. In the second approach, a full vector model is used, and the poles and mode shapes are estimated in one step by solving an eigenvalue problem. Both of the models assume 14 modes, some of them are considered non-physical. Results for the 8 most significant modes in the estimations are compared.

Nomenclature

i	: Counting integer
n, m	: Integers, order of ARMA model
k	: Integer, discrete time
M	: Integer, number of channels
N	: Integer, number of modes
t, τ	: Continuous time
Δt	: Sampling interval
$y_c(t)$: Continuous response
$Y(t)$: Continuous stochastic process
Φ_i, Θ_i	: AR and MA parameters
$y_i(k)$: Discrete response time series
σ_i	: Standard deviations of responses
$e(k)$: White noise time series
λ_i	: Poles
f_i	: Eigenfrequency
ζ_i	: Damping ratio
C	: Covariance matrix
$R(\tau)$: Continuous covariance function
d_i	: Covariance weight factors
P_i	: Modal Participation factors
$R(k)$: Discrete covariance function matrix

$y(k), e(k)$: Vectors of response and load
$x(k), a(k)$: State vectors of response and load
ψ_i	: Eigenvectors of state-space model
D_i	: Covariance weight matrices

Introduction

For offshore platforms and other structures subjected to natural loads like wave and wind, the loads are usually unknown, and thus, a traditional modal analysis based on frequency response functions, Ewins [1], cannot be performed. Usually, the analysis must be based on measurements of the response only. A review of the problems of identifying the dynamic properties of offshore structures is given in Jensen [2].

It is usually assumed that the loads might be modelled as stationary white noise. This assumption however, does not imply any serious limitations. It is possible to show, Ibrahim et al. [3], that if the load process is not white noise, but might be modelled by a set of second order differential equations, i.e. modelled as the response of a mechanical system loaded by white noise, then the structure might be identified using an oversized model to incorporate the non-physical modes describing the load process. This idea is used in the present analysis.

Auto Regressive Moving Average (ARMA) models have been used mainly in electrical engineering, Ljung [4], Söderström, & P. Stoica [5] and in economics, Harvey [6]. In the last decade, however, the use of ARMA models for identification of structural systems has become more common, Piombo et al. [7], Kozin & Natke [8], Safak [9]. The main advantages of these techniques are that they are accurate for extracting information from noisy signals, that estimation of the parameter covariance matrix is a natural part of the estimation, and that they can be formulated as exact covariance equivalent discrete time domain models of a set of second order differential equations, Pandit [10], Andersen et al. [11].

The main aim of this investigation is to illustrate the applicability of ARMA models on a real case: a multi-

channel measurement of the response of a wave loaded offshore structure. The analysed time series are relatively short, and thus, the modelling of the noise is essential for extracting reliable information about modal parameters.

Two approaches are presented and used for estimation of modal parameters. First, a scalar ARMA model is used to estimate eigenfrequencies and damping ratios. Then this information is used for fitting an analytical form of the covariance functions to the empirically estimated covariance function to obtain the mode shapes. The second approach is based on a more general ARMA model that is formulated directly for multi-channel output systems. Using this approach, the estimation is made in one step solving an eigenvalue problem. Results from the two approaches are compared.

Test Case

The offshore platform is a multi-pile structure located in Lake Maracaibo, Venezuela, and it houses the power plant for a large oil production complex for the Venezuelan oil industry.

The platform consists of a reinforced base structure and a steel superstructure which holds the power generation equipment. This platform was built in 1992, and it experiences continuous vibrations caused by wave and current actions which are to be evaluated in order to determine the effects of this continuous movement on the integrity of the structure.

The reinforced concrete structure is 58 m (195') long and 20 m (66') wide, it is supported by 42 pre-stressed concrete piles, 0.91 m (36") in diameter, 55 m (185') long, precast and driven on site, see figure 1. The steel-framed structure holds 5 turbo-generators and their control rooms.

Water depth at the location of the offshore platform is about 30 m. Wave heights in this zone have been reported to vary between 1.20 m and 2.50 m, with recurrence periods of 3.8 s and 4.9 s respectively. The information reported for current action near the platform shows values in the order of 1.4 m/s in the direction of the waves.

To measure vibrations of the platform due to ambient excitation, the structure was instrumented using 8 seismic accelerometers of the DC type with a maximum frequency of 3500 Hz. The accelerometers were placed at 4 points at the topside deck as shown in figure 1. Signals were amplified 100 times, and low-pass filtered at 5 Hz before sampling at a sampling frequency of 12.8 Hz. Data were recorded simultaneously at all 8 channels using a multi-channel data acquisition system converting from analog to digital with 16-bit accuracy. Data were recorded in blocks with 8192 data points per channel corresponding to a recording time of about 10 min.

Before the data were used for modal analysis, the data were filtered and decimated. Since the physical modes were known to be around 1 Hz, the signals were decimated with a factor 4 reducing the data blocks to 2048 points per channel and the Nyquist frequency to 1.60 Hz.

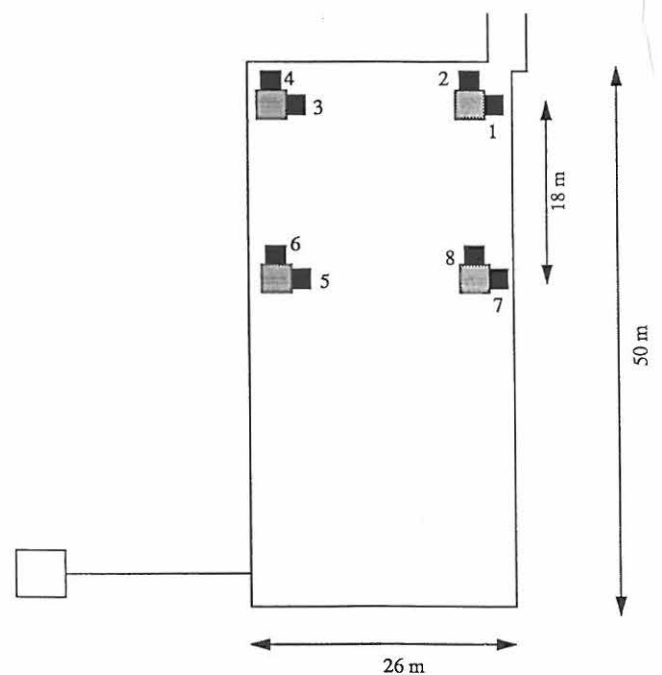


Figure 1. Test arrangement on the offshore platform. The figure shows the measurement points on the topside deck.

Before decimating, the signals were digitally low-pass filtered to reduce noise. Since the signals had a large long-periodic component, the signals were also digitally high-pass filtered at a cut-off frequency of 0.032 Hz.

Since the accelerometer number 3 some times showed unreliable results, this channel was excluded in the analysis. The modal analysis reported in this paper was based on the data from one data block only, i.e. on 7 channels, each with 2048 data points.

Estimation by Scalar ARMA Model

Let $y_c(t)$ be a realisation of the continuous stochastic process $Y(t)$, and let $y_c(t)$ have the discrete (sampled) representation $y(k) = y_c(k\Delta t)$, where Δt is the sampling interval. An ARMA model of the order (n, m) , called ARMA(n, m), for the time series $y(k)$ is then given by

$$y(k) = \sum_{i=1}^n \Phi_i y(k-i) - \sum_{i=1}^m \Theta_i e(k-i) + e(k) \quad (1)$$

where Φ_i are the auto regressive (AR) parameters describing the response $y(k)$ as a linear regression on the past values, and Θ_i are the moving average (MA) parameters describing the response $y(k)$ as a linear regression on the past values of an unknown time series, $e(t)$. Now, since the response $y(k)$ might be considered as a linear regression of the past responses and the past unknown loads, the last term $e(k)$ in eq. (1) is called the residue, since it might be considered as the term describing the deviation of the measured time series $y(k)$ from the response predicted by the regression. Thus using minimum least squares, the best fit corresponds to minimising the variance of the residue $e(k)$.

It might be shown, that an ARMA model of order $(2N, 2N-1)$ is the covariance identical discrete model of a continuous system with N degrees of freedom, Pandit [10], Kozin et al. [8], Andersen et al. [11].

When the AR parameters are known, the modal parameters are found from the $2N$ roots λ of the characteristic AR polynomial, Pandit [10]

$$\lambda^{2N} - \Phi_1 \lambda^{2N-1} - \dots - \Phi_{2N-1} \lambda - \Phi_{2N} \quad (2)$$

These roots are called the poles. The roots of the similar characteristic MA polynomial are called the zeroes. The poles and the zeroes are complex numbers. Zeroes and poles lying outside the unit circle correspond to unstable systems, and thus, usually zeroes and poles are forced inside the unit circle. Problems with unstable zeroes and poles usually relates to problems with too many degrees of freedom. The poles corresponding to physical degrees of freedom always appear in complex conjugate pairs, one pair for each degree of freedom. The eigenfrequencies f_i and damping ratios ζ_i are found from the relation between the modal parameters and the N complex conjugate poles

$$\lambda = \exp(2\pi i f \Delta t (-\zeta \pm i \sqrt{1 - \zeta^2})) \quad (3)$$

Since the covariance matrix \mathbf{C}_θ of all the estimated AR and MA parameters $\theta = \{\theta_1, \theta_2, \dots\}^T$ is easily obtained in the estimation process, Ljung [4], the covariance matrix \mathbf{C}_a of any set of physical parameters $\mathbf{a} = \{a_1, a_2, \dots\}^T$ might be estimated by linearisation, see e.g. Kirkegaard [12]

$$\mathbf{C}_a = \mathbf{G} \mathbf{C}_\theta \mathbf{G}^T \quad (4)$$

where \mathbf{G} is a gradient matrix describing the linear relationship of \mathbf{a} around its mean value $\mathbf{a} = \boldsymbol{\mu}_a + \mathbf{G}\theta$, $\boldsymbol{\mu}_a = E[\mathbf{a}]$. This technique is used to calculate standard deviations of the estimated eigenfrequencies and damping ratios.

As explained earlier, not all of the estimated modes might correspond to structural modes. A helpful tool in judging to what extent an estimated mode might be physical or not, is to have a measure of its significance

for the response. If a certain mode dominates the response, then it is reasonable to assume it to correspond to a structural mode - or to a dominating frequency of the loading system. On the other hand, if the contribution to the response is little, the estimated mode might be describing some non-physical phenomenon. A useful measure might be obtained using that the auto covariance function $R(\tau) = E[Y(t - \tau)Y(t)]$ can be written as a weighted sum of oscillators, one oscillator for each mode, Pandit [10]

$$R(k\Delta t) = \sum_{i=1}^N d_i \lambda_i^k \quad (5)$$

Now, using an unbiased estimate of $R(\tau)$, e.g. by using the unbiased FFT, see e.g. Bendat & Piersol [13] or Brincker et al. [14], and the poles λ_i estimated by the ARMA model, the weights d_i might be estimated by least square regression. Now, since the poles appear in complex conjugate pairs, so does the weights d_i . Thus, for each oscillator, the energy content is proportional to $|d|$, and, thus, $P = \sqrt{|d|}$ is a measure of the modal amplitude. The participation factors P_i are determined for each estimated mode, and the participation vector is normalised to length one.

For the present multi-channel case, the time series for each channel were stacked to form one long record. In order to weight all channels equally, the time series were normalised to variance one before the merging, and in order to make a smooth transition between the merged time series to prevent transients in the residue $e(k)$, the time series were tapered before high-pass filtering.

Figure 2 shows the variance of the residue and Akaike's Final Prediction Error (FPE) for the different scalar ARMA models estimated for the system. A small variance of the residue indicates a good fit, thus, the smaller values of the variance, the better the fit. However, in order not to over-parameterise the problem, Akaike's Final Prediction Error (FPE), that expresses a statistical trade-off between the variance of the residue, and the number of model parameters, is considered. As it appears from the figure, models were tested with 1 to 15 degrees of freedom. The jump from 5 to 6 degrees of freedom and the flat curve for higher values, indicate that at least 5 physical modes are present (one mode is used to model the high-pass filter). Since the ARMA(28,27) model was the model with the lowest FPE, this model was chosen for the further analysis. The results for the eigenfrequencies and damping ratios with estimated standard deviations and the participation factors for each mode are given in table 1.

The results are shown graphically in figure 3 showing the spectrum and correlation of the residue, a plot comparing the FFT spectrum with the spectrum estimated by the ARMA model, and finally a plot showing zeroes and poles with 99 % confidence regions. The results indicate, that the residue is close to white noise, thus, all the information of the response signals is extracted by

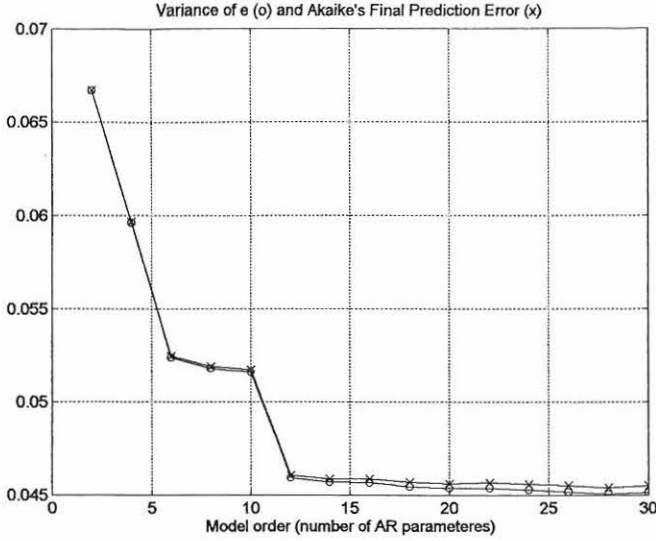


Figure 2. The deviation between models and measured time series expressed as the variance of the residue $e(k)$ and Akaike's Final Prediction Error (FPE) as a function of the number of Auto Regressive (AR) parameters.

the ARMA estimation, that the model gives a good fit, and that all parameters are reasonably well estimated.

To estimate the mode shapes, the response of the different channels $y_i(k)$ must be considered separately. Now, let $R_{ij}(\tau) = E[Y_i(t - \tau)Y_j(t)]$ be the covariance functions, and let them constitute the covariance function matrix $\mathbf{R}(k)$ with the elements $R_{ij}(k\Delta t)$. Then an expression similar to eq. (5) might be derived, Pandit [10],

$$\mathbf{R}(k) = \sum_{i=1}^N \mathbf{D}_i \lambda_i^k \quad (6)$$

If only one mode is present, then all channels would be fully correlated, and the mode shapes would simply be given by the standard deviations σ_i of the responses, and the sign of the mode shape (the phase) would be given by the correlations (± 1). Thus, the mode shapes would be given by the rows or the columns of the covariance matrix $\mathbf{C}_y = \mathbf{R}(0)$. Since eq. (6) is the modal decomposition of the covariance function matrix, the modal weight matrices \mathbf{D}_i play a similar role in the multiple degree of freedom case.

The weight matrices were found by estimating the covariance function matrix by the unbiased FFT, and then using least square linear regression. Once the weight matrices are estimated, the rows or the columns constitute a set of estimates for the mode shapes. In this case, only the 8 most significant modes were used for estimation of the modal weight matrices, and the mode shapes were estimated as the normalised mean of the

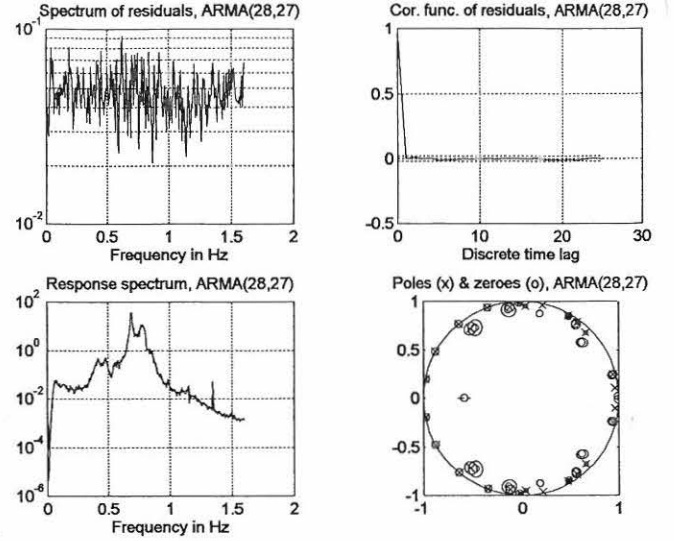


Figure 3. Graphical presentation of the results of fitting a scalar ARMA(28,27) model. Upper figures indicate that the residuals is approximately white noise. Lower figures indicate a good fit with reasonable uncertainty on all model parameters.

columns of the weight matrices. The results are shown in figure 4 together with the modal complexity factor MCF1 and MCF2, Imregun and Ewins [15]. The mode shapes were estimated on the assumption that no longitudinal strain occurs between the measurement points, shear strain however, was allowed. Only one mode is nearly free of complexity, mode 4 at $f = 0.69$ Hz. All other modes suffer a relatively high complexity. The complexity might be due to for instance non-linearities or non-proportional damping, in this case however, the most likely reason is noise and estimation errors.

Estimation by ARMAV model

The ARMAV model is a vector ARMA model. The responses $y(k)$ for the channels $i = 1, 2, \dots, M$ are organised in the vector $\mathbf{y}(k) = \{y_1(k), y_2(k), \dots\}^T$, and the ARMAV(n, m) model is a straightforward generalisation of the scalar case

$$\mathbf{y}(k) = \sum_{i=1}^n \Phi_i \mathbf{y}(k-i) - \sum_{i=1}^m \Theta_i \mathbf{e}(k-i) + \mathbf{e}(k) \quad (7)$$

In this case, the AR coefficients Φ_i and the MA coefficients Θ_i are (generally full) $M \times M$ matrices. In the estimation process, Ljung [4] suggest to minimize the determinant of the covariance matrix of the noise time series $\mathbf{e}(k)$. Once the model is estimated, the modal parameters are extracted by formulating the corresponding

Table 1. Results from scalar ARMA model (28,27)

Mode	Frequency f_i (Hz)	Standard deviation σ_{f_i} (Hz)	Damping ratio ζ_i (%)	Standard deviation σ_{ζ_i} (%)	Participation factor P_i
1	0.0580	0.0010	26.58	2.07	0.0402
2	0.1309	0.0062	10.70	5.28	0.0247
3	0.4080	0.0038	7.11	0.91	0.1379
4	0.4806	0.0037	2.87	0.78	0.0850
5	0.5439	0.0038	2.45	0.69	0.0858
6	0.6905	0.0007	0.97	0.11	0.6753
7	0.7802	0.0038	3.31	0.49	0.6507
8	0.8076	0.0029	0.90	0.36	0.2338
9	0.8641	0.0084	3.15	0.93	0.1509
10	0.9825	0.0006	0.15	0.06	0.0181
11	1.1232	0.0127	5.28	0.99	0.0734
12	1.1549	0.0003	0.06	0.03	0.0277
13	1.3438	0.0000	0.00	0.00	0.0198
14	1.4989	0.0012	0.13	0.08	0.0227

Table 2. Results from ARMAV(4,3)

Mode	Frequency f_i (Hz)	Damping ratio ζ_i (%)	Participation factor P_i
1	0.0577	100.0	0.0245
2	0.1938	62.7	0.0551
3	0.2495	79.0	0.0418
4	0.3707	22.6	0.0918
5	0.3905	37.9	0.0789
6	0.4008	9.6	0.0960
7	0.6034	100.0	0.0473
8	0.6956	1.3	0.7330
9	0.7008	17.5	0.0626
10	0.7721	1.64	0.4890
11	0.7924	1.66	0.4312
12	1.0382	18.1	0.0153
13	1.1190	36.9	0.0423
14	1.3029	33.1	0.0143

discrete time state space model

$$\mathbf{x}(k) = \Phi \mathbf{x}(k-1) + \Theta \mathbf{a}(k) \quad (8)$$

where the state vector $\mathbf{x}(k)$ and the load vector $\mathbf{a}(k)$ are made by stacking the responses and the noise vectors in the following way,

$$\mathbf{x}(k) = \begin{bmatrix} \mathbf{y}(k) \\ \mathbf{y}(k-1) \\ \mathbf{y}(k-2) \\ \vdots \end{bmatrix}; \quad \mathbf{a}(k) = \begin{bmatrix} \mathbf{e}(k) \\ \mathbf{e}(k-1) \\ \mathbf{e}(k-2) \\ \vdots \end{bmatrix} \quad (9)$$

and the Auto regressive matrix of the state space model is given by

$$\Phi = \begin{bmatrix} \Phi_1 & \Phi_2 & \cdot & \cdot & \cdot & \Phi_n \\ \mathbf{I} & 0 & \cdot & \cdot & \cdot & 0 \\ 0 & \mathbf{I} & & & & 0 \\ \cdot & & & & & \cdot \\ \cdot & & & & & \cdot \\ 0 & & & & \mathbf{I} & 0 \end{bmatrix} \quad (10)$$

For an ARMAV(2N, 2N-1) with M channels, the model is a covariance equivalent discrete model of a continuous system with NM degrees of freedom, Andersen et al. [11]. For this case, the vectors $\mathbf{x}(k)$ and $\mathbf{a}(k)$ both have the length NM , and the autoregressive matrix Φ of the state space model is $NM \times NM$. The poles and the mode shapes are found by solving the eigenvalue problem that naturally arises from eq. (8)

$$(\lambda_i \mathbf{I} - \Phi) \psi_i = 0 \quad (11)$$

the eigenvalues λ_i constitute the poles, and in this formulation, the mode shapes are the last M components

of the eigenvectors ψ_i . Similar to the scalar case, the eigenvalues and eigenvectors appear in complex conjugate pairs, one pair for each degree of freedom.

For direct comparison of the two ARMA approaches, an ARMAV model was chosen with the same number of degrees of freedom as for the scalar case, thus, an ARMAV(4,3) was estimated for the 7 channels corresponding to 28 poles or 14 degrees of freedom. When the ARMAV model is known, the weight matrices \mathbf{D}_i of the covariance function matrix is easily determined, Andersen et al. [11], and a participation factor might be defined by taking a scalar measure of \mathbf{D}_i . In this investigation the modal participation factors P_i were estimated by taking the square root of the sum of the eigenvalues of \mathbf{D}_i . Again, the modal participation vector was normalised to length one.

The results are shown in table 2 giving eigenfrequencies, damping ratios and participation factors for the 14 modes. An uncertainty measure might be estimated using the covariance matrix for the estimated parameters. This was not done however, for the vector ARMA case. The eight most significant modes were selected using the participation factors, though excluding mode 6 ($f = 0.6034$ Hz) due to the unrealistically large damping. The eight mode shapes are shown in figure 5.

As it appears from the results, again the mode shape has a relatively large degree of complexity, except the mode at $f = 0.69$ Hz. Comparing the mode shapes with the mode shapes estimated by the scalar ARMA approach it appears that the mode shape at 0.69 Hz is very much the same, whereas some modes at 0.40 - 0.41 Hz, 0.77 - 0.78 Hz are relatively close, the mode shapes at 1.12 Hz is quite different, and the rest of the modes do not seem to correspond. The conclusion is, that the most dominant mode at 0.69 Hz is determined with a good accuracy concerning both eigenfrequency and mode shape, two modes are determined with a good accuracy for the eigenfrequency, and somewhat larger uncertainty on the mode shape, and one mode is determined only with respect to eigenfrequency.

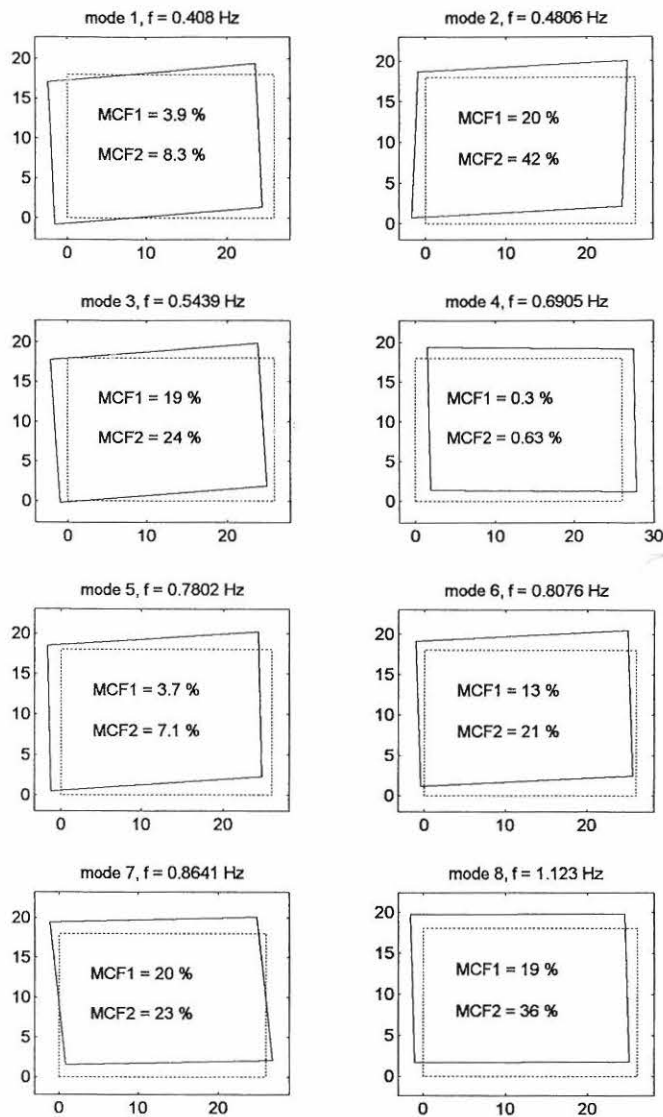


Figure 4. Mode shapes estimated by the scalar ARMA approach determining the modal weight matrices for the covariance function by fitting an analytical form to the empirical covariance function matrix.

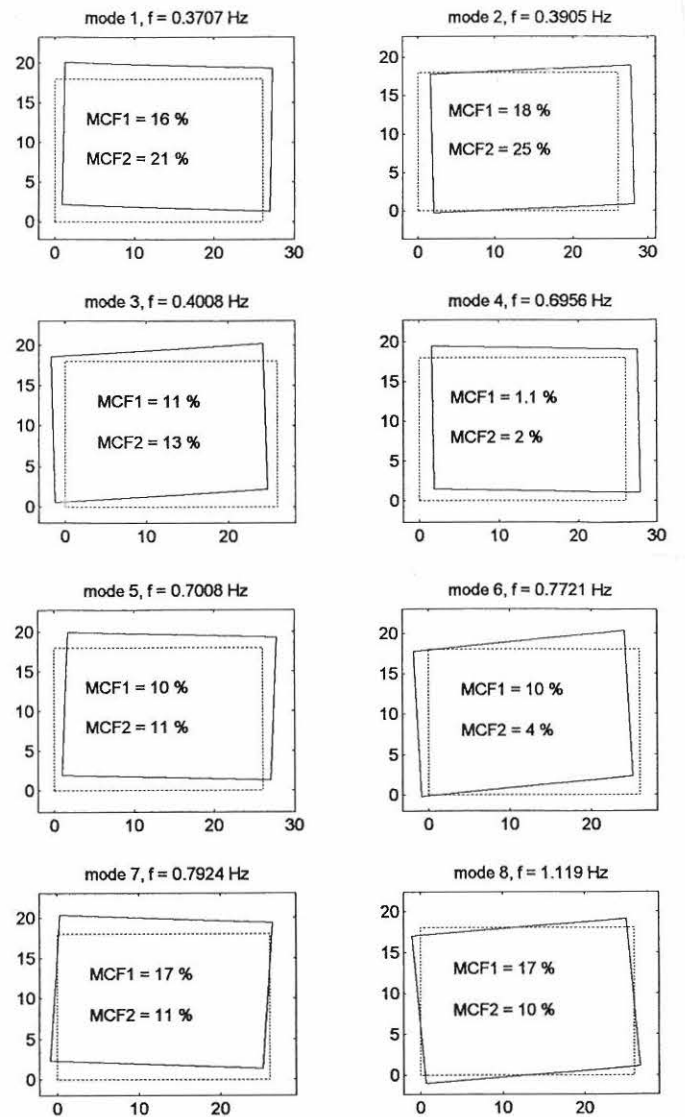


Figure 5. Mode shapes estimated by using an ARMAV model and solving the eigenvalue problem arising from the state space formulation of the model.

Comparing the differences between the estimated values of eigenfrequencies with the uncertainty measure from the scalar approach, it seems like the standard deviations underestimate the uncertainty. This might be due to bias errors.

An important difference between the results of the two models is, that the ARMAV model estimates three significant close modes, whereas the scalar model only estimates two. The mode at 0.78 Hz in the scalar model

is split into two modes at 0.77 and 0.79 Hz. Since the ARMAV model takes advantage of cross information between channels, and thus should be a stronger tool in detecting close modes, and since the mode is split into two modes with a smaller damping ratio around 1.6 %, the results of the ARMAV model could be a better estimate of the structural behaviour. On the other hand, since the complexity of the modes did not reduce, it can only be concluded, that the possibility of two close modes exists.

Conclusions

Two ARMA approaches have been compared for estimating eigenfrequencies, damping ratios and mode shapes for an operating offshore platform loaded basically by sea waves. The acceleration response was measured at 7 points at the topside deck, and the responses were analysed without using any information about the load.

Comparing the two approaches, it seems that because of the larger freedom in choosing the degrees of freedom in the model, the scalar model has an advantage in a better understanding of the necessary degrees of freedom needed to have a good estimate. Further, the scalar model gives relatively good estimates of both eigenfrequencies, damping ratios and mode shapes, although the ARMAV model is believed to be better in detecting close modes. The accuracy in determining the modal parameters seems to be comparable for the two approaches. The two models agree quite well on the most significant modes, concerning both eigenfrequencies, damping ratios and mode shapes. The damping ratio of the dominant mode (0.69 Hz) was estimated as 1.3 % for the ARMAV model and 0.9 % for the scalar approach.

Acknowledgements

This work was carried out under a grant from the Danish Technical Research Council. This support as well as support from INTEVEP, S.A. Research and Technology Center for Petróleos de Venezuela, and from the Department of Building Technology and Structural Engineering at Aalborg University, Denmark is gratefully acknowledged.

References

- [1] Ewins, D.J.: *Modal Testing: Theory and Practice*. Research Studies Press Ltd., John Wiley and Sons, 1991.
- [2] Jensen, J.L.: *System Identification of Offshore Structures*. Ph.D.-Thesis, Aalborg University, 1991.
- [3] Ibrahim, S.R., R. Brincker and J. Asmussen: *Modal Parameter Identification from Responses of General Unknown Random Inputs*. Proc. of the 14th International Modal Analysis Conference (IMAC), Dearborn, Michigan, 1996.
- [4] Ljung, Lennart: *System Identification - Theory for the User*. Prentice-Hall, Inc., 1987.
- [5] Söderström, T. & P. Stoica: *System Identification*. Prentice Hall, 1987.
- [6] Harvey, A.C.: *The Economical Analysis of Time Series*. Philip Allan Publishers, Ltd., Oxford, 1981.
- [7] Piombo, B., E. Giorcelli, L. Garibaldi and A. Fasana: *Structures Identification using ARMAV Models*. Proc. of the 11th International Modal Analysis Conference (IMAC), Kissimmee, Florida, 1993.
- [8] F. Kozin & H. G. Natke: *System Identification Techniques*. Structural Safety, Vol. 3, 1986.
- [9] Safak, E.: *Identification of Linear Structures using Discrete-Time Filters*. Journal of Structural Engineering, Vol. 117, No. 10, 1991.
- [10] Pandit, S.M.: *Modal and Spectrum Analysis: Data Dependent Systems in State Space*. John Wiley & Sons, Inc., 1991.
- [11] Andersen, P., R. Brincker and P.H. Kirkegaard: *Theory of Covariance Equivalent ARMAV Models of Civil Engineering Structures*. Proc. of the 14th International Modal Analysis Conference (IMAC), Dearborn, Michigan, 1996.
- [12] Kirkegaard, P.H.: *Optimal Design of Experiments for Parametric Identification of Civil Engineering Structures*. Ph.D.-Thesis, Aalborg University, 1994.
- [13] Bendat, J.S. and A.G. Piersol: *Random Data, Analysis and Measurement Procedures*. John Wiley & Sons, Inc., 1986.
- [14] Brincker, R., S. Krenk, P.H. Kirkegaard and A. Rytter: *Identification of Dynamical Properties from Correlation Function Estimates*. Danish Society for Structural Science and Engineering, Vol. 63, No. 1, 1992, pp. 1-38.
- [15] Imregun, M. and D.J. Ewins: *Complex Modes - Origins and Limits*. Proc. of the 13th International Modal Analysis Conference (IMAC), Nashville, Tennessee, 1995.

FRACTURE AND DYNAMICS PAPERS

PAPER NO. 36: J. P. Ulfkjær & R. Brincker: *Indirect Determination of the $\sigma - w$ Relation of HSC Through Three-Point Bending*. ISSN 0902-7513 R9229.

PAPER NO. 37: A. Rytter, R. Brincker & P. H. Kirkegaard: *An Experimental Study of the Modal Parameters of a Damaged Cantilever*. ISSN 0902-7513 R9230.

PAPER NO. 38: P. H. Kirkegaard: *Cost Optimal System Identification Experiment Design*. ISSN 0902-7513 R9237.

PAPER NO. 39: P. H. Kirkegaard: *Optimal Selection of the Sampling Interval for Estimation of Modal Parameters by an ARMA-Model*. ISSN 0902-7513 R9238.

PAPER NO. 40: P. H. Kirkegaard & R. Brincker: *On the Optimal Location of Sensors for Parametric Identification of Linear Structural Systems*. ISSN 0902-7513 R9239.

PAPER NO. 41: P. H. Kirkegaard & A. Rytter: *Use of a Neural Network for Damage Detection and Location in a Steel Member*. ISSN 0902-7513 R9245

PAPER NO. 42: L. Gansted: *Analysis and Description of High-Cycle Stochastic Fatigue in Steel*. Ph.D.-Thesis. ISSN 0902-7513 R9135.

PAPER NO. 43: M. Krawczuk: *A New Finite Element for Static and Dynamic Analysis of Cracked Composite Beams*. ISSN 0902-7513 R9305.

PAPER NO. 44: A. Rytter: *Vibrational Based Inspection of Civil Engineering Structures*. Ph.D.-Thesis. ISSN 0902-7513 R9314.

PAPER NO. 45: P. H. Kirkegaard & A. Rytter: *An Experimental Study of the Modal Parameters of a Damaged Steel Mast*. ISSN 0902-7513 R9320.

PAPER NO. 46: P. H. Kirkegaard & A. Rytter: *An Experimental Study of a Steel Lattice Mast under Natural Excitation*. ISSN 0902-7513 R9326.

PAPER NO. 47: P. H. Kirkegaard & A. Rytter: *Use of Neural Networks for Damage Assessment in a Steel Mast*. ISSN 0902-7513 R9340.

PAPER NO. 48: R. Brincker, M. Demosthenous & G. C. Manos: *Estimation of the Coefficient of Restitution of Rocking Systems by the Random Decrement Technique*. ISSN 0902-7513 R9341.

PAPER NO. 49: L. Gansted: *Fatigue of Steel: Constant-Amplitude Load on CCT-Specimens*. ISSN 0902-7513 R9344.

PAPER NO. 50: P. H. Kirkegaard & A. Rytter: *Vibration Based Damage Assessment of a Cantilever using a Neural Network*. ISSN 0902-7513 R9345.

PAPER NO. 51: J. P. Ulfkjær, O. Hededal, I. B. Kroon & R. Brincker: *Simple Application of Fictitious Crack Model in Reinforced Concrete Beams*. ISSN 0902-7513 R9349.

PAPER NO. 52: J. P. Ulfkjær, O. Hededal, I. B. Kroon & R. Brincker: *Simple Application of Fictitious Crack Model in Reinforced Concrete Beams. Analysis and Experiments*. ISSN 0902-7513 R9350.

FRACTURE AND DYNAMICS PAPERS

PAPER NO. 53: P. H. Kirkegaard & A. Rytter: *Vibration Based Damage Assessment of Civil Engineering Structures using Neural Networks*. ISSN 0902-7513 R9408.

PAPER NO. 54: L. Gansted, R. Brincker & L. Pilegaard Hansen: *The Fracture Mechanical Markov Chain Fatigue Model Compared with Empirical Data*. ISSN 0902-7513 R9431.

PAPER NO. 55: P. H. Kirkegaard, S. R. K. Nielsen & H. I. Hansen: *Identification of Non-Linear Structures using Recurrent Neural Networks*. ISSN 0902-7513 R9432.

PAPER NO. 56: R. Brincker, P. H. Kirkegaard, P. Andersen & M. E. Martinez: *Damage Detection in an Offshore Structure*. ISSN 0902-7513 R9434.

PAPER NO. 57: P. H. Kirkegaard, S. R. K. Nielsen & H. I. Hansen: *Structural Identification by Extended Kalman Filtering and a Recurrent Neural Network*. ISSN 0902-7513 R9433.

PAPER NO. 58: P. Andersen, R. Brincker, P. H. Kirkegaard: *On the Uncertainty of Identification of Civil Engineering Structures using ARMA Models*. ISSN 0902-7513 R9437.

PAPER NO. 59: P. H. Kirkegaard & A. Rytter: *A Comparative Study of Three Vibration Based Damage Assessment Techniques*. ISSN 0902-7513 R9435.

PAPER NO. 60: P. H. Kirkegaard, J. C. Asmussen, P. Andersen & R. Brincker: *An Experimental Study of an Offshore Platform*. ISSN 0902-7513 R9441.

PAPER NO. 61: R. Brincker, P. Andersen, P. H. Kirkegaard, J. P. Ulfkjær: *Damage Detection in Laboratory Concrete Beams*. ISSN 0902-7513 R9458.

PAPER NO. 62: R. Brincker, J. Simonsen, W. Hansen: *Some Aspects of Formation of Cracks in FRC with Main Reinforcement*. ISSN 0902-7513 R9506.

PAPER NO. 63: R. Brincker, J. P. Ulfkjær, P. Adamsen, L. Langvad, R. Toft: *Analytical Model for Hook Anchor Pull-out*. ISSN 0902-7513 R9511.

PAPER NO. 64: P. S. Skjærbæk, S. R. K. Nielsen, A. Ş. Çakmak: *Assessment of Damage in Seismically Excited RC-Structures from a Single Measured Response*. ISSN 1395-7953 R9528.

PAPER NO. 65: J. C. Asmussen, S. R. Ibrahim, R. Brincker: *Random Decrement and Regression Analysis of Traffic Responses of Bridges*. ISSN 1395-7953 R9529.

PAPER NO. 66: R. Brincker, P. Andersen, M. E. Martinez, F. Tallavó: *Modal Analysis of an Offshore Platform using Two Different ARMA Approaches*. ISSN 1395-7953 R9531.

**Department of Building Technology and Structural Engineering
Aalborg University, Sohngaardsholmsvej 57, DK 9000 Aalborg
Telephone: +45 98 15 85 22 Telefax: +45 98 14 82 43**