Measuring Undrained Shear Strength using CPT and Field Vane

Luke, Kirsten

Published in:

Publication date:
1992

Document Version
Publisher's PDF, also known as Version of record

Link to publication from Aalborg University

Citation for published version (APA):

General rights
Copyright and moral rights for the publications made accessible in the public portal are retained by the authors and/or other copyright owners and it is a condition of accessing publications that users recognise and abide by the legal requirements associated with these rights.

? Users may download and print one copy of any publication from the public portal for the purpose of private study or research.
? You may not further distribute the material or use it for any profit-making activity or commercial gain
? You may freely distribute the URL identifying the publication in the public portal

Take down policy
If you believe that this document breaches copyright please contact us at vbn@aub.aau.dk providing details, and we will remove access to the work immediately and investigate your claim.

Downloaded from vbn.aau.dk on: November 10, 2019
NGM-92

11. NORDISKE GEOTEKNIKERMØDE
AALBORG, 28-30 MAJ 1992

Vol 1 / 3

Artikler til NGM-92: Session 1-4

Maj 1992
dgf-Bulletin

DANISH GEOTECHNICAL SOCIETY
DANSK GEOTEKNISK FORENING
NGM-92
11. NORDISKE GEOTEKNIKERMØDE
AALBORG, 28-30 MAJ 1992

Vol 1 / 3

Artikler til NGM-92: Session 1-4

Maj 1992
dgf-Bulletin 9

DANISH GEOTECHNICAL SOCIETY
DANSK GEOTEKNISK FORENING
Measuring undrained shear strength using CPT and Field Vane

By

Kirsten Luke, Kampsax Geodan/Aalborg Universitetscenter

SUMMARY
This paper presents the results of CPT's and Field Vane tests from two small test areas with different soils, Glacial Till and Yoldia Clay. An average of $N_k = q_t/c_v$ for the Yoldia Clay is 7.7 with a standard deviation of 0.7. The average of $N_k$ for the Glacial Till is 9.7 with a standard deviation of 2.2.

INTRODUCTION
An attempt has been made to find a correlation between cone resistance $q_t$ and undrained shear strength in a simple linear form /1/.

$$q_t = N_k \cdot c_v$$

where $q_t$ = cone resistance. $N_k$ = cone factor. $c_v$ = undrained shear strength obtained by Field Vane.

Tests made elsewhere have produced a cone factor $N_k$ with a very wide range $5 < N_k < 35$. Usually when $q_t$ is related to $c_v$ there is only one or very few borings with vane tests to which the CPT results can be related. This might be one of the reasons for the large scatter observed.

This paper presents the results from two test sites in Denmark with different clays, Rørdal near Aalborg and Purhus near Randers. Once the test areas were marked out CPT's and Field Vane tests were performed as close to each other as possible in order to ensure reliability. The exact test positions at Rørdal can be seen on Fig. 1.

This large number of tests forms the basis for a statistical evaluation of $q_t$ and $c_v$, thereby making it possible to find an approximate average value for $N_k$ and an estimate of the variation of $N_k$ for these two soils.
Fig. 1: Test area at Rørdal. ● CPT (to depths of 2 to 3 meters). ✗ vane test at four depths 0.5, 1.0, 1.25, and 1.5 m.

TEST EQUIPMENT

CPT
The cone penetrometer used is the A.P. van den Berg piezocone. It measures cone resistance $q_c$, sleeve friction $f_s$, and pore pressure $u$. The cone has a 60° apex angle, cross sectional area of 10 cm², and a friction sleeve area of 150 cm². The pore pressure filter is located just behind the cone. The cone resistance is afterwards corrected for the pore pressure acting on the area behind the cone /3/ making the effective cone area 10 cm².

$$q_t = q_c + (1 - a)u$$

where $q_t$ = corrected cone resistance. $q_c$ = measured cone resistance. $u$ = measured pore pressure $a$ = effective area ratio (0.7 found by calibration for this specific cone).

The rate of penetration was 2 cm/sec. in all tests.

Field Vane
The Field Vane is the Danish type A vane with a diameter of 3.3 cm and height of 6.6 cm. The vane is hammered into the ground and the vane test is performed immediately afterwards. The maximum shear should be reached within and as near 2 min. as possible.

TEST SITES

Rørdal, Aalborg (Yoldia Clay)
The soil on this site was deposited in the ice sea which covered parts of northern Denmark about 10,000 years ago. The clay is rather fat, inorganic with very thin stripes (approximately 1 mm) of silt, sand, and chalk. The Yoldia Clay has been classified at three depths
0.5, 1.0 and 1.5 m. At each depth six samples were used to obtain the consistency limits. At least three samples were used for the estimation of the other classification properties. The average soil properties for each depth can be seen in Table 1.

<table>
<thead>
<tr>
<th>Depth [m]</th>
<th>d, [cm]</th>
<th>w [%]</th>
<th>w_L [%]</th>
<th>w_p [%]</th>
<th>I_p [%]</th>
<th>I_c [%]</th>
<th>Clay cont. [%]</th>
<th>Activity</th>
<th>e</th>
<th>γ [kN/m³]</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.5</td>
<td>2.78</td>
<td>31.1</td>
<td>62.7</td>
<td>26.3</td>
<td>36.4</td>
<td>0.87</td>
<td>58</td>
<td>0.63</td>
<td>0.86</td>
<td>19.2</td>
</tr>
<tr>
<td>1.0</td>
<td>2.71</td>
<td>29.6</td>
<td>59.6</td>
<td>24.7</td>
<td>34.8</td>
<td>0.85</td>
<td>59</td>
<td>0.59</td>
<td>0.87</td>
<td>19.2</td>
</tr>
<tr>
<td>1.5</td>
<td>2.76</td>
<td>33.3</td>
<td>65.1</td>
<td>26.2</td>
<td>38.9</td>
<td>0.82</td>
<td>62</td>
<td>0.63</td>
<td>0.90</td>
<td>19.1</td>
</tr>
</tbody>
</table>

Table 1: Classification properties for the Yoldia Clay in Rødal.

Purhus, Randers (Glacial Till)
The clay on this site is a Glacial Till, unsorted, described as yellowish-brown, sandy to very sandy, with no chalk, and gravelly. It is a deposit from the Weichsel ice 10,000 to 15,000 years ago. The soil properties were classified by "Statens Vejlaboratorium", Denmark, in a boring 10 meters from the test area. The results are listed in Table 2.

<table>
<thead>
<tr>
<th>Depth [m]</th>
<th>d, [cm]</th>
<th>w [%]</th>
<th>w_L [%]</th>
<th>w_p [%]</th>
<th>I_p [%]</th>
<th>I_c [%]</th>
<th>Clay cont. [%]</th>
<th>Activity</th>
<th>e</th>
<th>γ [kN/m³]</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.2 - 2.0</td>
<td>2.73</td>
<td>15.2</td>
<td>23.5</td>
<td>15.0</td>
<td>8.5</td>
<td>0.98</td>
<td>15</td>
<td>0.57</td>
<td>0.41</td>
<td>22.3</td>
</tr>
</tbody>
</table>

Table 2: Soil properties for the Glacial Till at Purhus, classified by Statens Vejlaboratorium.

**TEST RESULTS**

When comparing cone penetration tests and vane tests q<sub>T</sub> and c<sub>v</sub> are both assumed to represent the soil strength at a given point. The problem that always exists when comparing one test type to another is that the measurements are not obtained at exactly the same point. By assuming that the soil properties do not change discontinuously over the area, we can use an estimation technique called "Kriging" to estimate the values between the measured points. Kriging is a stochastic interpolation method that accounts for the uncertainties associated with soil properties /2/.

In the following Kriging is used to estimate the distribution of both q<sub>T</sub> and c<sub>v</sub> over the area. By dividing the q<sub>T</sub>-surface with the c<sub>v</sub>-surface, the variation of N<sub>k</sub> is found.
Fig. 2: $q_{i}/10$ (kPa); $c_v$ (kPa) and $N_k$ in four depths at Rørdal, 0.5 m, 1.0 m, 1.25 m, and 1.5 m.

<table>
<thead>
<tr>
<th></th>
<th>$q_{i}/10$ (kPa)</th>
<th>$c_v$ (kPa)</th>
<th>$N_k$</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>0.5m</td>
<td>1.0m</td>
<td>1.25m</td>
</tr>
<tr>
<td>$n$</td>
<td>20</td>
<td>20</td>
<td>19</td>
</tr>
<tr>
<td>$\bar{z}$</td>
<td>164</td>
<td>167</td>
<td>146</td>
</tr>
<tr>
<td>$s_v^2$</td>
<td>835</td>
<td>143</td>
<td>125</td>
</tr>
<tr>
<td>$s_s$</td>
<td>29</td>
<td>12</td>
<td>11</td>
</tr>
<tr>
<td>$m$</td>
<td>170</td>
<td>165</td>
<td>150</td>
</tr>
<tr>
<td>$v$</td>
<td>0.18</td>
<td>0.07</td>
<td>0.08</td>
</tr>
</tbody>
</table>

Table 3: Statistical properties for Yoldia Clay at Rørdal. $n =$ number of samples. $\bar{z} =$ sample mean. $s_v^2 =$ variance. $s_s =$ std. deviation. $m =$ median. $v =$ coefficient of variation.
Glacial Till

Fig. 3: $q_t/10$ (kPa); $c_v$ (kPa) and $N_k$ in four depths at Purhus, 0.5 m, 1.0 m, 1.25 m, and 1.5 m.

<table>
<thead>
<tr>
<th></th>
<th>$q_t/10$ (kPa)</th>
<th></th>
<th>$c_v$ (kPa)</th>
<th></th>
<th>$N_k$</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>0.5m</td>
<td>1.0m</td>
<td>1.25m</td>
<td>1.5m</td>
<td>0.5m</td>
</tr>
<tr>
<td>$n$</td>
<td>20</td>
<td>20</td>
<td>20</td>
<td>19</td>
<td>25</td>
</tr>
<tr>
<td>$ar{x}$</td>
<td>157</td>
<td>120</td>
<td>77</td>
<td>66</td>
<td>138</td>
</tr>
<tr>
<td>$s^2$</td>
<td>2928</td>
<td>1893</td>
<td>148</td>
<td>74</td>
<td>1527</td>
</tr>
<tr>
<td>$s_x$</td>
<td>54</td>
<td>44</td>
<td>12</td>
<td>9</td>
<td>39</td>
</tr>
<tr>
<td>$m$</td>
<td>135</td>
<td>105</td>
<td>75</td>
<td>65</td>
<td>131</td>
</tr>
<tr>
<td>$v$</td>
<td>0.34</td>
<td>0.37</td>
<td>0.16</td>
<td>0.14</td>
<td>0.28</td>
</tr>
</tbody>
</table>

Table 4: Statistical properties for the Glacial Till at Purhus. $n$ = number of samples. $\bar{x}$ = sample mean. $s^2$ = variance. $s_x$ = std. deviation. $m$ = median. $v$ = coefficient of variation.
DISCUSSION

CPT and Field Vane tests have been performed in two small test areas with different soil types, Glacial Till and Yoldia Clay.

The greatest standard deviations are measured in the upper layers probably due to crust formation in summer. The compaction of the clay depends on the capillary forces which vary much over the area.

The depth at which the capillary forces influence test results varies according to soil types, possibly 1.0 m in Glacial Till and 0.5 m in Yoldia Clay.

When not taking the test results in the crust zone into account the average of $N_k$ is approximately 7.7 for Yoldia Clay and 9.7 for Glacial Till. These values compare well with other $N_k$ values registered for similar soils, Yoldia Clay in Aalborg ($N_k = 8$) and Glacial Till in the Great Belt ($N_k = 10$) /1/. The standard deviation for $N_k$ in Yoldia Clay is 0.7 and 2.2 for $N_k$ in Glacial Till.

The coefficient of variation ($s_x/x$) which signifies the uniformity of the test results is 0.1 for $N_k$ in Yoldia Clay. The test results from Glacial Till show a coefficient twice that of Yoldia Clay (0.2) as can be expected due to lack of uniform consistency of Clay Till which contains both stones and gravel.

On the two test sites there is a tendency for $N_k$ to decrease with depth of penetration. The most likely explanations are:

1. The soil contains non-fully saturated clay because it lies above ground water level which is the case for the Clay Till examined at Purhus and the Yoldia Clay above 1 m depth at Rørdal. Yoldia Clay contains sand/silt stripes and Clay Till has a high sand content. It appears, therefore, that degree of saturation can affect $N_k$.

2. As the vane is hammered into the ground any friction on the vane rod will increase with depth penetration, thereby creating a higher $c_v$ value and consequently lower $N_k$ values.

REFERENCER


Nadim, F. (1988): Geotechnical Site Description using Stochastic Interpolation. NGI publikasjon 176. /2/
