CHAPTER 116

STOCHASTIC MODELLING OF THE DIFFUSION COEFFICIENT FOR CONCRETE¹

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ABSTRACT
In the paper, a new stochastic modelling of the diffusion coefficient \( D \) is presented. The modelling is based on a physical understanding of the diffusion process and on some recent experimental results. The diffusion coefficient \( D \) is strongly dependent on the w/c ratio and the temperature. A deterministic relationship between the diffusion coefficient and the w/c ratio and the temperature is used for the stochastic modelling. The w/c ratio and the temperature are modelled by log-normally and normally distributed stochastic variables, respectively. It is then shown by Monte Carlo simulation that the diffusion coefficient \( D \) may be modelled by a normally distributed stochastic variable. The sensitivities of \( D \) with regard to the mean values and the standard deviations are evaluated.

1. INTRODUCTION
Corrosion of the reinforcement is the major reason for deterioration of reinforced concrete structures in many countries. Modelling the corrosion process is very complex and it is often based on observations or speculations rather than a clear understanding of the physical and chemical processes behind the corrosion process.

Corrosion initiation period refers to the period of time during which the passivation of steel is destroyed and the reinforcement starts corroding actively. Fick’s law of diffusion can represent the rate of chloride penetration into concrete, as a function of depth from the concrete surface and as a function of time.

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\[
\frac{dC(x,t)}{dt} = D \frac{d^2C(x,t)}{dx^2}
\]

where \(C(x,t)\) is the chloride ion concentration, as % by weight of cement, at a distance of \(x\) m from the concrete surface after \(t\) seconds of exposure to the chloride source. \(D\) is the chloride diffusion coefficient expressed in \(\text{m}^2/\text{sec}\). If \(C_{cr}\) is assumed to be the critical chloride corrosion concentration and \(d\) is the thickness of concrete cover, then the corrosion initiation period \(T_{corr}\) can be calculated by

\[
T_{corr} = \frac{d^2}{4D} \left( \frac{1}{\Phi} \left( \frac{C_{cr} - C_0}{C_0 - C_i} \right) \right)^2
\]

where \(C_0\) is the equilibrium chloride concentration on the concrete surface, as % by weight of cement, \(\text{erf}\) is the error function.

It follows from (2) that the time to corrosion imitation is inversely proportional in \(D\). It is therefore of great interest to get a good estimate of \(D\). According to extensive experimental investigations by Jensen [1] and Jensen et al. [2]), it can be concluded that the most important factors are the water/cement ratio \(w/c\), the temperature \(\Phi\), and the amount of e.g. silica fume s.f. The experiments show that the diffusion coefficient \(D\) increases significantly with \(w/c\) as well as with the temperature \(\Phi\). The influence of \(w/c\) and the temperature \(\Phi\) may be explained by the chloride binding. Only the free chloride is important for the diffusion coefficient \(D\). With increased \(w/c\) ratio less chloride is bound and therefore \(D\) is increased. The strong influence of the temperature is mainly caused by thermal activation of the diffusion process, but may also be due to a reduced chloride binding when the temperature is increased. The purpose of the paper is to use the experimental results in Jensen [1], Jensen et al. [2]) to make an improved stochastic modelling of the diffusion coefficient \(D\), see also Thoft-Christensen [3], [4].

2 THE CORROSION PROCESS

In principle, reinforced concrete is an excellent type of structure from a corrosion point of view, since the alkaline environment in the concrete maintains a passive film on the surface of the reinforcement, and this film protects the reinforcement against corrosion. However, if the concrete is penetrated by e.g. water or carbon dioxide, then this passive film breaks down and the reinforcement is open to corrosion; Neville [5].

The chloride-induced corrosion is schematically illustrated in figure 1. An anodic region is established, where the passive film is broken down and an electrochemical cell is formed. The passive surface is the cathode, and the electrolyte is the pore water in the concrete. At the anode the following reactions take place:
Fe $\rightarrow$ Fe$^{++}$ + 2e$^-$

Fe$^{++}$+2(OH)$^-\rightarrow$ Fe(OH)$_2$

4Fe(OH)$_2$+2H$_2$O+O$_2$ $\rightarrow$ 4 Fe(OH)$_3$ (3)

Chloride ions Cl$^-\rightarrow$ activate the unprotected surface and form an anode. The chemical reactions are

Fe$^{++}$+2Cl$^-\rightarrow$FeCl$_2$

FeCl$_2$+2H$_2$O $\rightarrow$ Fe(OH)$_2$+2HCL (4)

It follows from (3) and (4) that two rust products Fe(OH)$_2$ and Fe(OH)$_3$ are produced. The different types of rust products are interesting to study because they have great influence on corrosion cracking, since the volume of the rust products corresponding to 1 cm$^3$ steel varies a lot; Nielsen [6].

<table>
<thead>
<tr>
<th>Corrosion product</th>
<th>Colour</th>
<th>Volume, cm$^3$</th>
</tr>
</thead>
<tbody>
<tr>
<td>Fe$_3$O$_4$</td>
<td>Black</td>
<td>2.1</td>
</tr>
<tr>
<td>Fe(OH)$_2$</td>
<td>White</td>
<td>3.8</td>
</tr>
<tr>
<td>Fe(OH)$_3$</td>
<td>Brown</td>
<td>4.2</td>
</tr>
<tr>
<td>Fe(OH)$_3$, 3H$_2$</td>
<td>Yellow</td>
<td>6.4</td>
</tr>
</tbody>
</table>

Table 1. Volume of corrosion products, from [6].

3. THE DIFFUSION COEFFICIENT

The diffusion coefficient $D$ is not a real physical constant for a given concrete structure since it depends on a number of factors. According to extensive experimental investigations (Jensen [1], Jensen et al. [2]) it can, as mentioned earlier, be concluded that the most important factors are the water/cement ratio w/c, the temperature $\Phi$, and the amount of e.g. silica fume s.f.

Figure 2 shows the diffusion coefficient $D$ as a function of the water-cement ratio w/c and of the temperature $\Phi$ C (Celsius).

Figure 2. The diffusion coefficient $D$ ($10^{-12}$ m$^2$/s) as a function of the water-cement ratio w/c and of the temperature $\Phi$ C (Celsius).
The addition of silica fume is of great importance for the chloride ingress. Silica fume additions reduce the chloride ingress because of changes in the pore structure, see table 2 (Jensen 1998).

<table>
<thead>
<tr>
<th>w/c</th>
<th>0.3</th>
<th>0.5</th>
</tr>
</thead>
<tbody>
<tr>
<td>% s.f.</td>
<td>0</td>
<td>3</td>
</tr>
<tr>
<td>D ((10^{-12} \text{ m}^2/\text{s}))</td>
<td>3.8</td>
<td>2.0</td>
</tr>
</tbody>
</table>

Table 2. The diffusion coefficient D as a function of % s.f. for two values of w/c and Φ = 20 °C.

The data above clearly indicate that site information is needed to make e.g. an estimation of the remaining life cycle or any estimation where the diffusion coefficient is involved. This has clearly been confirmed by several authors e.g. in (Suda et al. [7]), where important information of the distribution of the diffusion coefficient D in Japan is shown. Figure 4 shows the mean air temperature Φ and the w/c ratio in Japan. As
expected, the temperature is much higher (21-25°C) in the southern part of Japan than in the northern part (4-9°C). The w/c ratio has a smaller variation, but the highest ratios are in the Kanto area where also the temperature $\Phi$ is relatively high, figure 5.

Figure 4. Mean air temperature $\Phi$ and w/c ratio in Japan, figures 3 and 5 in Suda et al. [7].

Figure 7. Distribution of the diffusion coefficient $D$ (cm$^2$/year) in Japan, figure 15 in Suda et al. [7].

4. BINDING MODIFIED DIFFUSION LAW

Fick’s law for diffusion was used as early as 1970 for calculation of the diffusion coefficients for various concrete compositions; Collepardi et al. [8]. Since then, Fick’s law has been the basis for chloride ingress modelling, but a large number of
modifications have been introduced; Jensen [1]. Fick’s diffusion law has e.g. been modified taking into account binding of chloride by assuming that the free chloride follows Fick’s law; Jensen [1], Jensen et al. [2]). It is also assumed that the chloride binding follows a Freundlich isothermal equation (Luping [9]).

\[ c_b = \alpha \times c_f^\beta \]  

(5)

where \(c_b\) [mg/n-gel] is the bound chloride, \(c_f\) [mol Cl/l solution] and \(\alpha\) and \(\beta\) are empirical constants. The chloride binding significantly modifies the shape of the chloride ingress profiles as well as the calculated chloride diffusion coefficient \(D\). In figure 6 the principal difference between chloride ingress profiles with and without binding is shown. Without bonding the profile is strongly concave and with binding the profile is almost linear. Generally, measured profiles are almost linear so a modelling with binding seems to be a great improvement.

![Figure 6. Chloride ingress profiles.](image)

In Jensen [1] is presented an example that strongly supports this conclusion. A cement paste with \(w/c = 0.3\) and with no additives was exposed to chlorides for 30 days at 35\(^{\circ}\)C. Without binding the diffusion coefficient is \(1.5 \times 10^{-12}\) m\(^2\)/s and with binding included the diffusion coefficient for the free chloride is \(3.7 \times 10^{-12}\) m\(^2\)/s. If binding is included the description is substantially improved.

5. THE W/C RATIO

![Figure 7. Distribution of w/c measure in 522 thin sections by fluorescence microscopy (Concrete-experts).](image)

The w/c ratio for an existing concrete element may be estimated using Optical
Fluorescence Microscopy; (Jakobsen [10] and Concrete-experts [10]). Thin sections of the concrete are fluorescent impregnated and analysed under an optical microscope using a combination of a blue excitation filter and a yellow blocking filter. In fluorescent light the epoxy filled air voids and cracks then appear yellow. Cement paste appears as shades of green and aggregates black. The shade of green of the cement paste depends on the capillary porosity. A sample with a low w/c ratio appears dark green and a sample with high w/c ratio appears light green. These shades of green are used to estimate the w/c ratio by comparing with the colours of a standard cement pastes where the w/c ratios are known.

In figure 7 is shown the determination of the w/c ratio of precast railroad ties performed on 522 thin sections representing 127 ties. The average value is 0.42 and the standard deviation is 0.026 (the coefficient of variation is 6%).

6. THE TEMPERATURE $\Phi$

It is suggested to model the temperature $\Phi$ as a stochastic variable based on the temperature at the site of the structure. The modelling should take into account that the yearly mean temperature varies significantly from year to year. The data needed for the stochastic modelling are in most cases available from national meteorological institutions. In figure 8 is as example shown the yearly mean temperature in Denmark for the years 1873-2001; Cappelen [12]. Notice that the mean yearly temperature in this time period varies between below 6°C to over 9°C and that there has been a significant increase in the mean yearly temperature. These facts can be taken into consideration by weighting the estimated diffusion coefficient.

![Figure 8. Yearly mean temperatures for Denmark 1873-2001.](image)

7. STOCHASTIC MODELLING OF THE DIFFUSION COEFFICIENT $D$

Based on the experimental results presented in section 3, the following formula may be used to approximately describe the diffusion coefficient $D$ as a function of the w/c ratio and the temperature $\Phi$:

$$D = 11.146 - 31.025 \times \text{w/c} - 1.941 \times \Phi + 38.212 \times (\text{w/c})^2 + 4.48 \times \text{w/c} \times \Phi + 0.024 \times \Phi^2$$  \hspace{1cm} (6)

As an example assume that w/c is log-normally distributed LN(0.45,0.02) and that the temperature $\Phi$ is normally distributed N(10.0°C, 1.0°C). Then by crude Monte Carlo simulation (10,000 samples) it may be shown that the diffusion coefficient may be modelled by a normally distributed stochastic variable $N(8.11 \times 10^{-12} \text{ m}^2/\text{s},$
$1.11 \times 10^{-12} \, \text{m}^2/\text{s}$ that is with a coefficient of variation equal to 14%, see figure 9.

The diffusion coefficient $D$ is, as expected strongly sensitive to the mean values of w/c and $\Phi$ as shown in figure 10. In the diagram to the left, w/c is log-normally distributed LN(E[w/c],0.02) and the temperature $\Phi$ is normally distributed N(10.0°C , 1.0°C ) and E[D] is the expected value of $D$. In the diagram to the right, w/c is log-normally distributed LN(0.45,0.02) and the temperature $\Phi$ is normally distributed N(E[Temperature] , 1.0°C ).

Figure 9. PDF of the diffusion coefficient $D$ ($10^{-12}\,\text{m}^2/\text{s}$).

Figure 10. Sensitivity analysis with regard to the mean values of w/c and of the temperature.

Figure 11. Sensitivity analysis with regard to the standard deviations of w/c and of the temperature.

The standard deviation std[D] of the diffusion coefficient $D$ (but not the expected value E[D]) is sensitive to the standard deviations of w/c and $\Phi$ as shown in figure 11. In the diagram to the left, w/c is log-normally distributed LN(0.45,std[w/c]) and the temperature $\Phi$ is normally distributed N(10.0°C , 1.0°C ). In the diagram to the right, w/c is log-normally distributed LN(0.45,0.02) and the temperature $\Phi$ is normally distributed N(10.0°C , 1.0°C ).
distributed $\mathcal{N}(10^\circ C, \text{std}[\text{temperature}])$.

8. CONCLUSIONS

In the paper the importance of taking into account the site dependency of the diffusion coefficient is emphasized. The diffusion coefficient $D$ depends on several parameters. The two most important parameters seem to be the w/c ratio and the temperature. In the paper a stochastic modeling of $D$ based on recent experimental results is presented using crude Monte Carlo simulation. The dependency of the w/c ratio and the temperature is included.

9. ACKNOWLEDGEMENT

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10. REFERENCES
